

5452

5452

Form 504
U. S. DEPARTMENT OF COMMERCE
COAST AND GEODETIC SURVEY
DESCRIPTIVE REPORT
Type of Survey _____
Field No. _____ Office No. <u>5452</u>
LOCALITY
State <u>New York.</u>
General locality <u>New York City</u>
Locality <u>Manhattan &amp; Bronx</u>
<u>1936</u>
CHIEF OF PARTY
<u>J.C. Partington Jr. NAB</u>
LIBRARY & ARCHIVES
DATE _____

SUPPLEMENTAL T

5452  
5452

Form 504  
Rev. Dec. 1933  
DEPARTMENT OF COMMERCE  
U.S. COAST AND GEODETIC SURVEY  
R. S. PATTON, DIRECTOR

DESCRIPTIVE REPORT

Air Photo  
Topographic  
~~Hydrographic~~

Sheet No. T-5452 ~~6-15-34~~

State New York

LOCALITY

New York City

Manhattan & Bronx

1936

CHIEF OF PARTY

J.C. Partridge - Jr. H. & G.E.

5452

5452  
SUPPLEMENTAL T

applied to Chart 746 - Mar 30, 1937 L.M.Z.  
applied to Chart 226 - Nov 16, 1937 L.M.Z.

1.  
DEPARTMENT OF COMMERCE  
U.S. COAST AND GEODETIC SURVEY

REG. NO.

TOPOGRAPHIC TITLE SHEET

The Topographic Sheet should be accompanied by this form, filled in as completely as possible, when the sheet is forwarded to the Office.

Field No. 84 & 85 **T5452**

REGISTER NO. T-5452 & ~~T-5453~~

State New York

General locality New York City

Locality Bronx and Manhattan

Scale 1: 5000 photographs Nov. 25, 1934 and Mar. 26, 1935  
Date of survey 19

Vessel Air Photo Compilation Party

Chief of party J.C. Partington  
*J.C. Partington*

Surveyed by See STATISTICS SHEET, page 2 of this report.

Inked by See STATISTICS SHEET, page 2 of this report.

Heights in feet above --- to ground to tops of trees

Contour, Approximate contour, Form line interval ---feet

Instructions dated March 14, 1934

Remarks: -----  
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\* STATISTICS \*

on

COMPILATION, FIELD NOS. 84 & 85, REGISTER NOS. T-5452 ~~T-5453~~

PHOTOGRAPH NO.	DATE	TIME	TIDE			
			High		Low	
			Time	Ht.	Time	Ht.
351-356 (876 A-8)	Nov.25, 1934	1:15 PM	11:19 AM	3.9	5:07 AM	0.3
			- -	- -	5:54 PM	0.0
✓333-337 (876 A-8)	Nov.25, 1934	11:04 AM	12:32 AM	4.1	6:17 AM	0.3
			12:29 PM	4.8	7:04 PM	0.0
338-340 (876 A-8)	Nov. 25, 1934	11:10 AM	11:50 AM	5.8	8:09 AM	0.8
			1:43 PM	6.2	8:46 PM	0.2
467-469 (876 A-8)	Mar.26, 1935	10:35 AM	1:33 AM	5.2	8:01 AM	0.4
			1:56 PM	4.1	7:33 PM	0.5

	By	Date	
		From	To
SCALE FACTOR (1.000)	R.C.Bolstad	(Previously determined)	
PROJECTION	W.E.Hackett	12-21-34	
PROJECTION CHECKED	J.P.O'Donnell	12-21-34	
CONTROL PLOTTED	W.E.Brown	3-8 -35	
CONTROL CHECKED	J.G.Albert	3-11-35	
SMOOTH RADIAL LINE PLOT	W.Barasch	6-29-35	7-12-35
RADIAL LINE PLOT CHECKED	J.C.Partington	May 1936	
DETAIL INKED	R.H.Young	7-20-36	8-31-36
PRELIMINARY REVIEW OF SHEET	J.C.Partington	9-1-36	9-10-36
	J.C.Partington	9-11-36	9-19-36
AREA OF DETAIL INKED (land area)	6.3	Square Statute Miles	
AREA OF DETAIL INKED (shoals)	0.0	"	"
LENGTH OF SHORELINE (more than 200 M. from opposite shore)	8.2	Stat. Miles	
LENGTH OF SHORELINE (rivers & sloughs less than 200 M.wide)	4.4	Stat. Miles	
LENGTH OF STREETS, ROADS, RAILROADS, TRAILS	251	Stat. Miles	
GENERAL LOCATION	New York City		
Location	Manhattan		
DATUM	North American 1927		
STATION Saint Johns, 1932 (N.Y.)	Latitude	40° 48' 18.214"	561.8 m.
	Longitude	73 57 47.186	1106.0 m.

(Adjusted computations)

## COMPILERS REPORT

for

AIR PHOTO TOPOGRAPHIC SHEET, FIELD NOS. 84 & 85; REGISTER NOS. T-5452 & ~~T-5453~~GENERAL INFORMATION

The Air-photo Field Inspection Report for Metropolitan New York attached to the descriptive report of AIR-PHOTO TOPOGRAPHIC SHEET, Field No. 90, Register No. T-5458, furnished the necessary information for the compilation of this sheet.

This sheet has been compiled from single lens photographs listed on the previous page of this report. Photographs numbers 351 to 356 (876 A-8) were taken on Nov. 25, 1934 at approximately two hours after high water. Photographs numbers 333 to 337 (876 A-8) were taken on Nov. 25, 1934 at approximately one and one-half hours before high water. Photographs numbers 338 to 340 (876 A-8) were taken on Nov. 25, 1934 at approximately two and one-half hours before high water at Lawrence Point. Photographs numbers 467 to 469 (876 A-8) were taken on Mar. 26, 1935 at approximately two and one-half hours after low water at Horns Hook.

The photographs were taken by the U.S. Army Air Corps at Mitchell Field, L.I., N.Y. with a special camera recently developed by the Fairchild Camera Corporation, 62-10 Woodside Ave., Woodside New York City. Due to the fact that these photographs were among the first to be taken by this camera, mechanical troubles were encountered which caused considerable difficulty at first. This probably accounts for the irregular time interval between exposures which in turn affects the overlap. This is also probably the cause of excessive tilt in some pictures. The camera is known as the "K-7C" by the Army and as the "K-7A" by the Fairchild Corporation. *See opposite page.*

The Army plane was piloted by Lieut. Cullen at an altitude very close to 15,000 feet; the photographer was Sergeant Cates. A 24 inch cone (focal length 24") was used with this camera, producing the negatives on a scale of 1: 7,500. Contact prints were furnished the field party for use in field inspection. The original negatives were used by the Washington office of the U.S. Coast and Geodetic Survey for enlarging a set of office prints on a scale of 1: 5,000. The 1: 5,000 prints were furnished the field party and were used in compiling this sheet.

CONTROL(a) Sources

Control for the compilation of this sheet was obtained from the following sources:

1. Triangulation, 1930-33 by R.W. Woodworth.
2. Triangulation, 1903-08, Greater New York.
3. U.S. Engineers stations as described on Form 524 submitted with this report.
4. 3 point theodolite fix. (Station "Mount Morris Park Flag Pole"). Attached to this report.
5. Triangulation, 1885 by G. Bradford (House of Refuge).  
*Destroyed by erection of Tri-borough Bridge.*

(b) Errors.

No error was found in the position of any of the triangulation stations located by the U.S.Coast and Geodetic Survey.

*within probable error of the photo plot.*

All of the U.S.Engineers stations were found to be correct except two. The two stations found to be in error are: Station 43 (U.S.E.) and West 139th St.(U.S.E.). The coordinate position of station 43 (U.S.E.) was found to disagree from the position as found by the radial plot by 7.0 meters in azimuth 297° (from north). The spotting of this station has been verified and the radial line plotted position verified. It has been located by four "cuts" which give a strong angle of intersection. The black  $2\frac{1}{2}$  mm circle shown on the celluloid sheet is the radial line plot position of this station. The coordinate position of West 139th St.(U.S.E.) was found to differ from the radial line plot position by 2.0 meters in azimuth 75° (from north). The spotting of this station on the photographs is somewhat open to question due to indefinite detail in the vicinity. It has been located by three "cuts" which give a strong angle of intersection. The black  $2\frac{1}{2}$  mm circle shown on the celluloid sheet is the radial line plot position of this station.

The geographic positions of all of the U.S.Engineers stations were computed from their coordinates and plotted on the sheet with a  $2\frac{1}{2}$  mm circle. They were all found to agree with the radial line plot within 1.0 meter, except the two mentioned above. All of the U.S.Engineers stations shown on this sheet are described on Form 524 and form a part of this report. Other recoverable objects are shown by the same  $2\frac{1}{2}$  mm. circle but are not described on Form 524.

Some of the U.S.Engineers stations which were found by the field inspection party are not shown on this sheet because they are not permanently marked. An effort has been made to show the most permanently marked stations at intervals of about one-half mile along the waterfront.

COMPILATION.(a) Method.

The following streets were tied in to existing triangulation stations by field measurements:

Amsterdam Ave.  
Fifth Ave.  
Second Ave.  
First Ave.  
116th Street

These streets were held fixed on the compilation which materially aided the radial line plot.

The usual radial line method of plotting was used in the compilation of this sheet together with the field measurements given in the preceding paragraph.

The U.S.Engineers stations were used as supplementary control and their positions accepted only after it was found that they agreed with the radial line plot.

(b) Adjustment of Plot.

No great difficulty was encountered in running the radial line plot, and no unusual adjustment of the plot was necessary, there is sufficient overlap between successive photographs and between adjacent flights to give a strong radial line plot. The pictures have very little scale fluctuation or tilt.

The only part of the sheet where the plot is not strong is along the meridian  $73^{\circ} 55'$  between latitude  $40^{\circ} 48' 30''$  and  $40^{\circ} 49' 30''$ . The radial points in this area are near the outer edge of the photographs but are located by 3 "cuts" and are believed to be correct within 2 meters in position. The junction between this sheet and compilation T-5089 is in agreement.

(c) Interpretation.

No attempt has been made to show the street railway systems. Only railroad tracks and elevated tracks have been shown. No information on the railroad track layouts on this sheet were available and they have been detailed entirely from the photographs. It is quite possible that some of the tracks have been omitted particularly in the railroad yards and sidings.

The double full line is used to show first class roads and streets (curb to curb), and the double dashed line is used to show second class roads, poor motor roads and walks in park areas.

An attempt has been made to show all of the buildings along the waterfront. Some of the more important buildings farther inland have also been shown. The stereoscope has been used freely in interpreting the shape of the buildings.

The usual graphic symbols were used and no difficulty was experienced in interpreting the photographic detail.

(d) Information from Other Sources.

At the time the photographs were taken there were numerous buildings on Randalls Island. Since that time the buildings have been razed and the Tri-borough Bridge and other changes have been built. The location of the Tri-borough Bridge and it's approaches in the Bronx and Manhattan have been taken from a celluloid contact of control survey T-6489a, executed in August 1936.

There is a discrepancy in the high water line between sheet T-6489a and this compilation in the vicinity of Bronx Kill at the abutments of the Triborough Bridge. Unless there have been changes in the high water line since Nov. 25, 1934 (when the photographs were taken) the shoreline is believed to be correctly shown on this compilation, T-5453. The descriptive report of T-6489a makes no mention of having surveyed the high water line. *Compilation corrected to T-6489a. B.G.d.*

T-6489a also shows changes in the highwater line near Latitude  $40^{\circ} 47.8'$  Longitude  $73^{\circ} 55.8'$ . The descriptive report also mentions this. The high water line on this compilation (T-5453) probably is incorrect at the present time due to the construction of the East River Drive. The shoreline on this compilation is taken from the photographs in the absence of more recent information.

*Compilation corrected to T-6489a.*

*B.G.d.*



Bridge Data - Clearance values of bridges on this compilation in Harlem River above and including Willis Ave. are from the Coast Pilot as stated on the opposite page. These values agree with the U.S.E. Bridge List for 1935, except:

Vertical clearances are listed in Bridge Book as above H.W. instead of M.H.W. as shown on this compilation. These actually are equal to the low water clearances listed in the Bridge Book - mean range of tide.

A number of the bridges have slightly different horizontal clearances on opposite sides of the center pier. Only the lower value is shown on the compilation as in the Coast Pilot.

The Triborough Bridge and Railroad Bridge clearances below Willis Ave. are from the U.S.E. Bridge List for 1935. M.H.W. values are shown are equal to Engineers Bridge List M.L.W. values - mean range of tide. The following data from the Engineers Bridge List is not shown on the compilation because of apparent errors in the list:

- (1) Triborough Bridge over Bronx Kills: Horizontal clearances given in Bridge Book disagree with blueprint 29320 of Triborough Bridge authority. Bridge not on photos of this area.
- (2) Railroad Bridge over Bronx Kills listed in Bridge Book as Bascule -- appears to be fixed as seen on photographs. 7
- (3) Triborough Bridge over Little Hell Gate - Horizontal clearances given in Bridge List do not agree with the blueprint 29318 of Triborough Bridge authority. Bridge does not appear on the photos.

The recent planetable survey of Triborough Bridge does not give clearance data.

(e) Names.

A list of the geographic names shown on this sheet are given on Form M 234 included with this report.

Street names may be taken from Map of the City of New York, Board of Estimate and Apportionment.

LIST OF RECOVERABLE OBJECTS.

Nine cards Form 524 are included with this report which describe the U.S. Engineers stations shown on this sheet.

The following stations were located by the radial line plot for use as recoverable objects. They are not described.

NAME	LATITUDE	LONGITUDE	METHOD OF DETERMINATION
North Twin Chy.	40° 48' 1690.5 m	73° 56' 183.0 m.	Radial plot.
Black Stack	40 48 313.5 m	73 55 449.0 m.	" "

Mount Morris Park Flag Pole was located by a 3 Pt. theodolite fix. This station is described on form 524 included with this report together with computations.

Six other stations located by the survey of sheet T-6489a are shown on this compilation. These are the two towers at the toll bridge on Randall's Island and the lights on top of the four supports to the lift span across the Harlem River.

All recoverable objects are shown with a  $2\frac{1}{2}$  m m. black circle.

BRIDGES:

The bridge data shown on the overlay sheet was taken from the Coast Pilot, Atlantic Coast, Section B, 1933; page 264. This data was compared with the publication "List of Bridges over the Navigable Waters of the United States", 1927; and no errors were found.

No data is on hand in this office pertaining to the clearances of the Tri-borough Bridge. This data probably can be obtained from the blue-prints of the Tri-borough Bridge Authority.

*See opposite page.*

Junctions.

This sheet joins compilation T-5451 to the northward along parallel 40° 49' 30" and the junction is satisfactory.

This sheet joins compilation T-5089 (1: 10,000 scale) to the northeastward along meridian 73° 55' 00". This sheet has been compared with a photostat enlargement of T-5089 and the junction appears to be satisfactory. No direct comparison of the two sheets has been made because sheet T-5089 was transmitted to the Washington office before this sheet was compiled.

This sheet joins compilation T-5458 (1: 5000 scale) at the south end of Randalls Island and the junction is satisfactory.

This sheet joins compilations T-5454 and T-5455 to the southward along 95 th Street and the junction is satisfactory.

#### COMPARISON WITH OTHER SURVEYS

No comparison between this sheet and other surveys has been made due to the fact that no other surveys of this area are on hand at this office. The discrepancies between this sheet and T-6489 are discussed on page 5 paragraph (d) of this report.

#### COMPARISON WITH CHARTS.

Due to the fact that the charts of this area are on a 1: 10,000 scale and the compilation is on a 1: 5000 scale no direct comparison between the two has been made.

However a comparison between this compilation and the 1: 5000 scale insert of chart 226 shows discrepancies of as much as 5 meters in the shoreline of the south side of Randalls Island. There are also discrepancies of as much as 17 meters in the shoreline of Sunken Meadow which appears to be shown too large on chart 226.

A comparison between this compilation and charts 226 and 274 show discrepancies in the shoreline of the north side of Bronx Kill in the vicinity of the railroad bridge and the Tri-borough Bridge.

The shoreline is believed to be correctly shown on this compilation and it is recommended that it be used in preference to that shown on the present charts.

#### RECOMMENDATIONS FOR FURTHER SURVEYS.

This sheet is believed to have a probable error of not greater than 2 meters in position for well defined detail of importance for charting, and not more than 4 meters for other detail. It is understood the width of roads, railroads, and similar detail may be slightly exaggerated in order to keep the detail clear when the sheet is reproduced.

On page 5 paragraph (d) of this report reference is made to the probable change in shoreline near Latitude  $40^{\circ} 47.8'$  Longitude  $73^{\circ} 55.8'$ . Except for this area this sheet is believed to be complete in all details of importance for charting and no additional surveys are required.

*A better estimate of the accuracy would be 0.2 to 0.6 mm for intersected points and 0.2 to 1.0 mm for other detail.*  
B.C.V.

Submitted by

*J.C. Partington*  
J.C. Partington  
Chief of Party

## Remarks

## Decisions

1		
2		
3	* Called "Bronx Kills" on local map	
4		
5		
6		
7		
8	* Called "N.Y.C.R.R." on field print	
9	* Called "Thomas Jefferson Park" on local map	
10		
11		
12		
13		
14		
15	Railroad yards nearby called "Manhattanville Term- * inal" on local map.	
16		
17		
18		
19		
20	* Called "Borough of Manhattan" on local map	
21		
22		
23		
24		
25		
26		
27		

# GEOGRAPHIC NAMES

Survey No. T-5452 & T-5453

GEOGRAPHIC NAMES		Survey No. T-5452 & T-5453									
Name on Survey	Inspection Party										
	On Chart No.	On previous survey No.	On U. S. quadrangle Maps	From local information	On local Maps	P. O. Guide or Map	Rand McNally Atlas	U. S. Light List			
A	B	C	D	E	F	G	H	K			
Hudson River ✓	746 ✓		✓	x	x					1	
Harlem River ✓	274 ✓		✓	x	x					2	
Bronx Kill ✓	226 ✓ 274				*					3	
Little Hell Gate ✓	226 ✓ 274				x					4	
Sunken Meadow ✓	226 ✓ 274				x					5	
Randalls Island ✓	226 ✓ 274		Randall I.		x					6	
N.Y.N.H. & H. R.R.	226 274				x					7	
N.Y.C. & H.R. R.R.	746 226 ✓			*						8	
Jefferson Park ✓	274			x	*					9	
Mount Morris Park ✓				x	x					10	
St. Nicholas Park ✓				x	x					11	
Morningside Park ✓					x					12	
Riverside Park ✓	746				x					13	
Central Park ✓	369 ✓		✓		x					14	
Manhattanville ✓	746 ✓		✓		*					15	
Columbia University					x					16	
College of the City of N.Y.					x					17	
Mott Haven ✓	274 ✓		✓		x					18	
Mott Haven Canal ✓	274 ✓									19	
New York City	369 746 ✓				*					20	
Borough of the Bronx					x					21	
Hebrew Orphan Asylum				x	x					22	
The Pool ✓					x					23	
The Loch ✓					x					24	
Harlem Meer ✓					x					25	
Names underlined in red approved by K.T.A. on 11/17/36										26	
Note: The local map under column "E" is called Map of the City of New York - Board of Estimate and Apportionment.										27	
										M 234	



**\$60,000,000 BRIDGE**  
An airplane view of the Triborough Bridge, joining Manhattan, the Bronx and Queens, which was dedicated July 11 by President Roosevelt  
(Wide World)





# NEW

A new interchange which solved a problem similar to that confronting Baltimore engineers who plan to empty traffic from the Howard street extension into North avenue at grade (Associated Press)

Note The two small islands  
and the pile shown on this  
compilation lat  $40^{\circ} 48.05'$  Long  $73^{\circ} 55.6'$   
at West entrance to Bronx  
kill have been added to  
the compilation in this office.  
These require field inspection  
for absolute identification but  
have been added in this  
office as they are ~~too close~~  
~~are~~ clearly fixed objects.

B.G. Jones



REVIEW OF AIR PHOTO COMPILATION T-5452  
Scale 1:5,000

Comparison with Graphic Control Surveys

1. T-6489a and b (August 1936), 1:5,000 - T-6489a and b is a location of the Triborough Bridge and adjacent detail which was constructed since the photos for this compilation and for compilation T-5458 were taken. All detail on T-6489a and b is shown on this compilation and on compilation T-5458.

There are no new hydrographic surveys in this area.

Comparison with Previous Topographic Surveys

Changes in this area have been so numerous that differences between this compilation and the old surveys are not listed in detail. The compilation is complete and adequate to supersede the sections of the following older topographic surveys which it covers, except for location of offlying rocks and contours, as listed below:

- T- 258 (1848), 1:5,000 - Except for rocks awash northeast of Sunken Meadow and on the south side of Randalls Island
- T- 475 (1854), 1:10,000
- T- 485 (1854), "
- T- 488 (1855), "
- T- 604 (1859), " - Except for contours and rocks at lat.  $40^{\circ} 48'$ , long.  $73^{\circ} 55.7'$
- T- 608 (1857), "
- T- 675 (1857), 1:5,000 - Except for the rocks in Bronx Kills
- T-1668 (1885), " , (combined hydro. and topo. survey) - Except for rocks in Bronx Kills, northeast of Sunken Meadow, and in Little Hell Gate.
- T-1703 (1886), 1:5,000 - Except for rocks in Bronx Kills
- T-1743 (1886), 1:10,000 - Except for contours along Hudson River

The contours listed on the above sheets are generally incomplete and conditions have no doubt been considerably changes by cuts and fills of construction work in this area. The rocks mentioned above are also discussed under the comparison with the charts.

Comparison with the Charts

1. Chart 226 - The rocks awash on chart 746 at lat.  $40^{\circ} 48'$ , long.  $73^{\circ} 55.7'$ , the numerous rocks and small islands in Bronx Kills, and the rocks awash in Little Hell Gate and around Sunken Meadow do not show on the photographs and are not shown on this compilation. None of these rocks are disproved by the photographs and their positions as shown on the chart are not affected by this compilation. In Bronx Kills the fact that the rocks above high water cannot be seen on the photos, while not disproving the existence of the rocks, indicates that they are not so large or prominent as shown on the chart.

*See note on opposite page also*

This compilation does not show all of the new construction on Randalls Island. Since the photos were taken all old buildings have been razed and new construction is now in progress (reported by graphic control survey T-6489, August 1936).

The dock on Randalls Island, lat.  $40^{\circ} 47.8'$ , long.  $73^{\circ} 55.6'$ , is gone except for piling remains.

2. Chart 274 - The wreck at lat.  $40^{\circ} 48.5'$ , long.  $73^{\circ} 56.1'$  on chart 274 is not shown on this compilation. The photographs show a number of barges moored over this position indicating that the wreck has been removed.

3. Chart 746 - The piling shown on chart 746 at lat.  $40^{\circ} 49.5'$ , long.  $73^{\circ} 57.5'$  is not visible on the photos and was not indicated by the field inspection. However, the photographs do not disprove the existence of broken-off remains slightly above water or under water.

The vessel at the Naval reservation is apparently permanently moored and in use and is shown on the compilation by solid lines.

This compilation shows numerous minor changes in water line detail and additions and changes to inshore detail on all of the above mentioned charts.

All landmarks including lights shown on the charts are on the compilation. No additional landmarks have been recommended by the compilation.

#### General

The control for this compilation is adequate and well distributed.

The compilation to be complete should have been supplemented by a planetable survey of the rocks mentioned above and definite statements should have been furnished by field inspection regarding existence or non-existence of the piling on chart 746 discussed above.

The projection on this compilation was hand drawn and a number of the lines were found out of position some 0.3 mm. These lines have been corrected as far as practicable before applying the grid system.

With the above exceptions the compilation is complete and carefully made as regards both the photo plotting and drafting.

Nov. 10, 1936.

*H. H. Schleiter*  
B. G. JONES

*B. G. Jones*

REVIEW OF AIR PHOTO COMPILATION NO. T-5452 ~~and 5453~~

Chief of Party: J.C.Partington

Compiled by: See STATISTICS

Project: HT-175

Instructions dated: Mar. 14, 1934.

- ✓ 1. The charts of this area have been examined and topographic information necessary to bring the charts up to date is shown on this compilation. (Par. 16a, b, c, d, e, g and 1; 26; and 64)
- ✓ 2. Change in position, or non-existence of wharfs, lights, and other topographic detail of particular importance to navigation which affect the chart, is discussed in the descriptive report. (Par. 26; and 66 g, n) *See review report preceding page.*
- ✓ 3. Ground surveys by plane table, sextant, or theodolite have been used to supplement the photographic plot where necessary to obtain complete information, and all such surveys are discussed in the descriptive report. (Par. 65; and 66 X, e)  
Five streets tied to existing triangulation - Page 4, Descriptive Report  
Tri-borough Bridge taken from Topographic Sheet T-6489a.  
3 pt. theodolite fix included with this report.
- ✓ 4. Blue-prints and maps from other sources which were transmitted by the field party contain sufficient control for their application to the charts. (Par. 28)  
  
No blue-prints transmitted.
- ✓ 5. Differences between this compilation and contemporary plane table and hydrographic surveys have been examined and rectified in the field before forwarding the compilations to the office and are discussed in the descriptive report.  
no contemporary surveys for comparison except Sheet T-6489a.
- ✓ 6. The control and adjustment of the photo plot are discussed in the descriptive report. Unusual or large adjustments are discussed in detail and limits of the area affected are stated. (Par. 12b; 44; and 66 c, h, i)  
  
Discussed in descriptive report.
- ✓ 7. High water line on marshy and mangrove coast is clear and adequate for chart compilation. (Par. 16a, 43, and 44)

NOTE: Strike out paragraphs, words or phrases not applicable and modify those requiring it. Paragraph numbers refer to those in the Topographic Manual. Refer also to the pamphlet "Notes on the Compilation of Planimetric Line Maps from Five Lens Air Photographs."

- ✓8. The representation of low water lines, reefs, coral reefs and rocks, and legends pertaining to them is satisfactory. (Par. 36, 37, ~~38~~, 39, 40, ~~41~~)
- ✓9. Recoverable objects have been located and described on Form 524 in accordance with circular 30, 1933, circular letter of March 3, 1933, and circular 31, 1934. (Par. 29, 30, and 57)
- ✓10. A list of landmarks was furnished on Form 567 and instructions in the Director's letter of July 16, 1934, Landmarks for Charts, complied with. (Par. 16d, e; and 60)  
No additional landmarks submitted.
- ✓11. All bridges shown on the compilation are accompanied by a note stating whether fixed or draw, clearance, and width of draw if a draw bridge. Additional information of importance to navigation is given in the descriptive report. (Par. 16c) ✓  
*No data on Tri-borough Bridge*
- ✓12. Geographic names are shown on the overlay tracing. The accepted local usage of new names has been determined and they are listed in the report, together with a general statement as to source of information and a specific statement when advisable. Complete discussion of place names differing from the charts and from the U. S. G. S. Quadrangles is given in the descriptive report, together with reasons for recommendations made. (Par. 64, and 66k) ✓
- ✓13. The geographic datum of the compilation is N.A. 1927 and the reference station is correctly noted. ✓
- ✓14. Junctions with adjoining compilations have been examined and are in agreement. (Par. 66j) ✓
- ✓15. The drafting is satisfactory and particular attention has been given the following:
- ✓1. Standard symbols authorized by the Board of Surveys and Maps have been used throughout except as noted in the report. ✓
  - ✓2. The degrees and minutes of Latitude and Longitude are correctly marked. ✓

- ✓3. All station points are exactly marked by fine ✓  
black dots.
- ✓4. Closely spaced lines are drawn sharp and clear ✓  
for printing.
- ✓5. Topographic symbols for similar features are of ✓  
uniform weight.
- ✓6. All drawing has been retouched where partially ✓  
rubbed off.
- ✓7. Buildings are drawn with clear straight lines ✓  
and square corners where such is the case on  
the ground.

(Par. 34, 35, 36, 37, ~~38~~, 39, 40, ~~41~~, 42, 43, 44, 45, 46, 48)

✓16. No additional surveying is recommended at this time. ✓

✓17. Remarks:

18. Examined and approved;

*J. C. Partington*  
Chief of Party

19. Remarks after review in office:

Reviewed in office by:

*B. G. Jones.*

Examined and approved:

*E. H. Green.*  
Chief, Section of Field Records  
*L. O. Robert.*  
Chief, Division of Charts

*Fred. L. Peacock*  
Chief, Section of Field Work  
*Stude*  
Chief, Division of Hydrography  
and Topography.



Station: Ken

State: Maryland

Chief of party: C. V. H.

Date: 1917

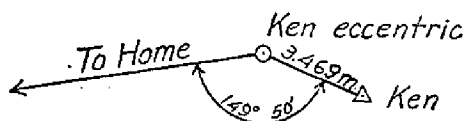
Computed by: O. P. S.

Observer: C. V. H.

Instrument: No. 163

Checked by: W. F. R.

OBSERVED STATION	Observed direction	Eccentric reduction	Sea level reduction	Corrected direction with zero initial	Adjusted direction
	° ' "	' "	"	° ' "	' "
Chevy .....	0 00 00.00	- 7.31		0 00 00.00	
Tank west of Δ Dulce .....	29 03 37.0	-1 09.8		29 02 34.5	
Ken (center), 3.469 meters .....	176 42				
Forest Glen standpipe .....	313 24 53.0	+3 01.2		313 23 01.5	
Home .....	326 31 30.21	+ 31.93		326 32 09.45	
Bureau of Standards, wireless pole .....	352 17 20.8	+ 5.7		352 17 33.8	
Reno .....	357 28 48.63	- 1.16		357 28 54.78	
Reference mark, 16.32 m .....	358 31 20				



This form, with the first three and fifth columns properly filled out and checked, must be furnished by field parties. To be acceptable it must contain every direction observed at the station.

It should be used for observations with both repeating and direction theodolites.

The directions at only one station should be placed on a page.

If a repeating theodolite is used, do not abstract the angles in tertiary triangulation. The local adjustment corrections (to close horizon only) are to be written in the Horizontal Angle Record, and the List of Directions is to be made from that record directly.

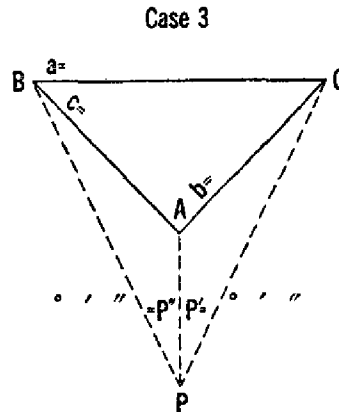
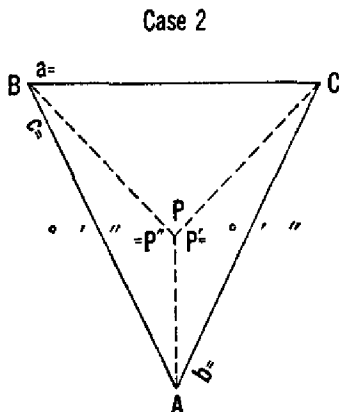
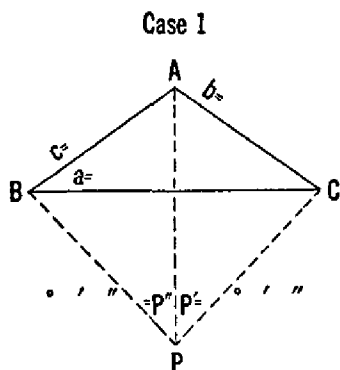
Choose as an initial for Form 24A some station involved in the local adjustment, and preferably one which has been used as an initial for a round of directions on objects not in the main scheme. Use but one initial at a station. Call the direction of the initial 0° 00' 00." 00, and by applying the corrected angles to this, fill in opposite each station its direction reckoned *clockwise* around the whole circumference regardless of the direction of graduation of the instrument. The clockwise reckoning is necessary for uniformity and to make the directions comparable with azimuths.

If a station has been occupied eccentrically, reduce to the center and enter in this form, in ink, the resulting corrections to the observed directions in the column provided for them. If an eccentric reduction is necessary, but not made in the field, leave the column blank. If the station was occupied centrally, and no eccentric reduction is required, put dashes in the column to show that no corrections are necessary.

Directions in the main scheme should be entered to hundredths of seconds in first-order triangulation; otherwise to tenths only. Points observed upon but once, direct and reverse, should be carried to tenths in first-order and second-order triangulation, and to even seconds only in third-order triangulation. In general, but two uncertain figures should be given.

It is recommended that the following simple plan of observing be used with a repeating instrument: Measure each single angle in the scheme at each station and the outside angle necessary to close the horizon. *Measure no sum angles.* Follow each measurement of every angle immediately by a measurement of its explement. Six repetitions are to constitute a measurement. The local adjustment will consist simply of the distribution of the error of closure of the horizon.

# COMPUTATION OF THREE-POINT PROBLEM



## Cases 1 and 2

P'	70	26	25.6'	✓
P''	73	09	19.2'	✓
A	149	03	03.6'	✓

Sum	292	38	48.4	✓
1/2 Sum	146	19	24.2	✓

$S = 180^\circ - \frac{1}{2} \text{sum} = 33 \quad 40 \quad 35.8$  ✓

Log c	=	3.4483160	✓
Log sin P'	=	9.9791864	✓
Colog b	=	6.3845620	✓
Colog sin P''	=	0.0190455	✓

Sum = log tan Z = 9.8261099 ✓

Z	=	33	49	27.2	✓
Z + 45°	=	78	49	27.2	✓

Log cot (Z + 45°) = 9.2957123 ✓

Log tan S = 9.8236888 ✓

Sum = log tan ε = 9.1193701 ✓

ε	=	7°	29'	58.2"	✓
S	=	33	40	35.8	✓

(Tan ε +)

S + ε = angle ABP	41	10	33.9	✓
S - ε = angle ACP	26	10	37.6	✓

BPA  
ABP  
PAB

APC  
PCA  
CAP

PCB  
CBP  
BPC

## Case 3

P'				
P''				
Sum				
A				

A - sum

$S = \frac{1}{2} (A - \text{sum}) =$

Dome (Grants Tomb)

N. Tower  
Eldorado

Mt. Morris Park  
flag pole ecc.

Highbridge



# INVERSE POSITION COMPUTATION

N.A. Datum

$$s_1 \sin \left( \alpha + \frac{\Delta \alpha}{2} \right) = \frac{\Delta \lambda_1 \cos \phi_m}{A_m}$$

$$s_1 \cos \left( \alpha + \frac{\Delta \alpha}{2} \right) = \frac{-\Delta \phi_1 \cos \frac{\Delta \lambda}{2}}{B_m}$$

$$-\Delta \alpha = \Delta \lambda \sin \phi_m \sec \frac{\Delta \phi}{2} + F(\Delta \lambda)^2$$

In which  $\log \Delta \lambda_1 = \log (\lambda' - \lambda)$  - correction for arc to sin\*;  $\log \Delta \phi_1 = \log (\phi' - \phi)$  - correction for arc to sin\*; and  $\log s = \log s_1 +$  correction for arc to sin\*.

		NAME OF STATION	
1. $\phi$	40° 48' 48.480	Dome (Grants Tomb)	73° 57' 48.370
2. $\phi'$	40° 47' 18.249	N. Tower (Eldorado)	73° 58' 04.044
$\Delta \phi (= \phi' - \phi)$	- 1 30.231	$\Delta \lambda (= \lambda' - \lambda)$	+ 0 15.674
$\frac{\Delta \phi}{2}$	- 45.116	$\frac{\Delta \lambda}{2}$	
$\phi_m (= \phi + \frac{\Delta \phi}{2})$	40° - 48' - 03.364		
$\Delta \phi$ (secs.)	- 90.231	$\Delta \lambda$ (secs.)	+ 15.674
$\log \Delta \phi$	1.955 3558	$\log \Delta \lambda$	1.195 1798
cor. arc - sin	-	cor. arc - sin	-
$\log \Delta \phi_1$	1.955 3558	$\log \Delta \lambda_1$	1.195 1798
$\log \cos \frac{\Delta \lambda}{2}$		$\log \cos \phi_m$	9.879 0869
$\text{colog } B_m$	1.489 2093	$\text{colog } A_m$	1.490 9019
$\log \{s_1 \cos (\alpha + \frac{\Delta \alpha}{2})\}$	3.444 5651 + (opposite in sign to $\Delta \phi$ )	$\log \{s_1 \sin (\alpha + \frac{\Delta \alpha}{2})\}$	2.565 1686 +
		$\log \{s_1 \cos (\alpha + \frac{\Delta \alpha}{2})\}$	3.444 5651 +
$\log \Delta \lambda$	1.195 1798	$\log \tan (\alpha + \frac{\Delta \alpha}{2})$	9.120 6035 +
$\log \sin \phi_m$	9.815 2011	$\alpha + \frac{\Delta \alpha}{2}$	7 31 12.28
$\log \sec \frac{\Delta \phi}{2}$		$\log \sin (\alpha + \frac{\Delta \alpha}{2})$	9.116 8521
$\log a$	1.010 3809	$\log \cos (\alpha + \frac{\Delta \alpha}{2})$	9.996 2485
a		$\log s_1$	3.448 3165
b		cor. arc - sin	+
$-\Delta \alpha$ (secs.)	+ 10.24	$\log s$	3.448 316
$\frac{\Delta \alpha}{2}$	5.12		
$\alpha + \frac{\Delta \alpha}{2}$	7 31 12.28		
$\alpha$ (1 to 2)	7 31 17.4		
$\Delta \alpha$	- 10.2		
	180		
$\alpha'$ (2 to 1)	187 31 07.2		

\* Use the table on the back of this form for correction of arc to sin.

NOTE.—For  $\log s$  up to 4.52 and for  $\Delta \phi$  or  $\Delta \lambda$  (or both) up to 10', omit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underscored, if using logarithms to 6 decimal places.

Table of arc-sin corrections for inverse position computations

$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	
4.177	1	2.686	5.223	124	3.732	5.525	497	4.034	
4.327	2	2.836	5.234	130	3.743	5.530	508	4.039	
4.415	3	2.924	5.243	136	3.752	5.534	519	4.043	
4.478	4	2.987	5.253	142	3.762	5.539	530	4.048	
4.526	5	3.035	5.260	147	3.769	5.543	541	4.052	
4.566	6	3.075	5.269	153	3.778	5.548	553	4.057	
4.599	7	3.108	5.279	160	3.788	5.553	565	4.062	
4.628	8	3.137	5.287	166	3.796	5.557	577	4.066	
4.654	9	3.163	5.294	172	3.803	5.561	588	4.070	
4.677	10	3.186	5.303	179	3.812	5.566	600	4.075	
4.697	11	3.206	5.311	186	3.820	5.570	613	4.079	
4.716	12	3.225	5.318	192	3.827	5.575	625	4.084	
4.734	13	3.243	5.326	199	3.835	5.579	637	4.088	
4.750	14	3.259	5.334	206	3.843	5.583	650	4.092	
4.765	15	3.274	5.341	213	3.850	5.587	663	4.096	
4.779	16	3.288	5.349	221	3.858	5.591	674	4.100	
4.792	17	3.301	5.356	228	3.865	5.595	687	4.104	
4.804	18	3.313	5.363	236	3.872	5.600	702	4.109	
4.827	20	3.336	5.369	243	3.878	5.604	716	4.113	
4.857	23	3.366	5.376	251	3.885	5.608	729	4.117	
4.876	25	3.385	5.383	259	3.892	5.612	743	4.121	
4.892	27	3.401	5.390	267	3.899	5.616	757	4.125	
4.915	30	3.424	5.396	275	3.905	5.620	771	4.129	
4.936	33	3.445	5.403	284	3.912	5.624	785	4.133	
4.955	36	3.464	5.409	292	3.918	5.628	800	4.137	
4.972	39	3.481	5.415	300	3.924	5.632	814	4.141	
4.988	42	3.497	5.422	309	3.931	5.636	829	4.145	
5.003	45	3.512	5.428	318	3.937	5.640	845	4.149	
5.017	48	3.526	5.434	327	3.943	5.644	861	4.153	
5.035	52	3.544	5.440	336	3.949	5.648	877	4.157	
5.051	56	3.560	5.446	345	3.955	5.652	893	4.161	
5.062	59	3.571	5.451	354	3.960	5.656	909	4.165	
5.076	63	3.585	5.457	364	3.966	5.660	925	4.169	
5.090	67	3.599	5.462	373	3.971	5.663	941	4.172	
5.102	71	3.611	5.468	383	3.977	5.667	957	4.176	
5.114	75	3.623	5.473	392	3.982	5.671	973	4.180	
5.128	80	3.637	5.479	402	3.988	5.674	989	4.183	
5.139	84	3.648	5.484	412	3.993	5.678	1005	4.187	
5.151	89	3.660	5.489	422	3.998				
5.163	94	3.672	5.495	433	4.004				
5.172	98	3.681	5.500	443	4.009				
5.183	103	3.692	5.505	453	4.014				
5.193	108	3.702	5.510	464	4.019				
5.205	114	3.714	5.515	474	4.024				
5.214	119	3.723	5.520	486	4.029				

# INVERSE POSITION COMPUTATION

N. A. Datum

$$s_1 \sin \left( \alpha + \frac{\Delta \alpha}{2} \right) = \frac{\Delta \lambda_1 \cos \phi_m}{\Delta m}$$

$$s_1 \cos \left( \alpha + \frac{\Delta \alpha}{2} \right) = \frac{-\Delta \phi_1 \cos \frac{\Delta \lambda}{2}}{B_m}$$

$$-\Delta \alpha = \Delta \lambda \sin \phi_m \sec \frac{\Delta \phi}{2} + F(\Delta \lambda)^3$$

in which  $\log \Delta \lambda_1 = \log (\lambda' - \lambda)$  - correction for arc to sin\*;  $\log \Delta \phi_1 = \log (\phi' - \phi)$  - correction for arc to sin\*; and  $\log s = \log s_1 +$  correction for arc to sin\*.

		NAME OF STATION	
1. $\phi$	40 48 48.480	Dome (Grants tomb)	73 57 48.370 ✓
2. $\phi'$	40 50 33.166	Highbridge	73 55 58.826 ✓
$\Delta \phi (= \phi' - \phi)$	+ 1 44.686	$\Delta \lambda (= \lambda' - \lambda)$	- 1 49.544
$\frac{\Delta \phi}{2}$	+ 52.343	$\frac{\Delta \lambda}{2}$	
$\phi_m (= \phi + \frac{\Delta \phi}{2})$	40 49 40.823		
$\Delta \phi$ (secs.)	+ 104.686	$\Delta \lambda$ (secs.)	- 109.544
$\log \Delta \phi$	2.0198886	$\log \Delta \lambda$	2.0395886
cor. arc-sin	-	cor. arc-sin	-
$\log \Delta \phi_1$	2.0198886	$\log \Delta \lambda_1$	2.0395886
$\log \cos \frac{\Delta \lambda}{2}$		$\log \cos \phi_m$	9.8789098
$\text{colog } B_m$	1.4892113	$\text{colog } A_m$	1.4909026
$\log \{s_1 \cos (\alpha + \frac{\Delta \alpha}{2})\}$	3.5090999 - (opposite in sign to $\Delta \phi$ )	$\log \{s_1 \sin (\alpha + \frac{\Delta \alpha}{2})\}$	3.4094010 -
		$\log \{s_1 \cos (\alpha + \frac{\Delta \alpha}{2})\}$	3.5090999 -
$\log \Delta \lambda$	2.0395886	$\log \tan (\alpha + \frac{\Delta \alpha}{2})$	9.9003011
$\log \sin \phi_m$	9.8154386	$\alpha + \frac{\Delta \alpha}{2}$	218 28 49.65
$\log \sec \frac{\Delta \phi}{2}$		$\log \sin (\alpha + \frac{\Delta \alpha}{2})$	9.7939633
$\log a$	1.8550272	$\log \cos (\alpha + \frac{\Delta \alpha}{2})$	9.8936621
a		$\log s_1$	3.6154377
b	- 71.62	cor. arc-sin	+
$-\Delta \alpha$ (secs.)		$\log s$	3.615438
$\frac{\Delta \alpha}{2}$	- 35.81		
$\alpha + \frac{\Delta \alpha}{2}$	218 28 49.65		
$\alpha$ (1 to 2)	218 28 13.8		
$\Delta \alpha$	+ 01 11.6		
	180		
$\alpha'$ (2 to 1)	38 29 25.4		

\* Use the table on the back of this form for correction of arc to sin.

NOTE.—For  $\log s$  up to 4.52 and for  $\Delta \phi$  or  $\Delta \lambda$  (or both) up to 10', omit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underscored, if using logarithms to 6 decimal places.

Table of arc-sin corrections for inverse position computations

$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	
4.177	1	2.686	5.223	124	3.732	5.525	497	4.034	
4.327	2	2.836	5.234	130	3.743	5.530	508	4.039	
4.415	3	2.924	5.243	136	3.752	5.534	519	4.043	
4.478	4	2.987	5.253	142	3.762	5.539	530	4.048	
4.528	5	3.035	5.260	147	3.769	5.543	541	4.052	
4.566	6	3.075	5.269	153	3.778	5.548	553	4.057	
4.599	7	3.108	5.279	160	3.788	5.553	565	4.062	
4.628	8	3.137	5.287	166	3.796	5.557	577	4.066	
4.654	9	3.163	5.294	172	3.803	5.561	588	4.070	
4.677	10	3.186	5.303	179	3.812	5.566	600	4.075	
4.697	11	3.206	5.311	186	3.820	5.570	613	4.079	
4.716	12	3.225	5.318	192	3.827	5.575	625	4.084	
4.734	13	3.243	5.326	199	3.835	5.579	637	4.088	
4.750	14	3.259	5.334	206	3.843	5.583	650	4.092	
4.765	15	3.274	5.341	213	3.850	5.587	663	4.096	
4.779	16	3.288	5.349	221	3.858	5.591	674	4.100	
4.792	17	3.301	5.356	228	3.865	5.595	687	4.104	
4.804	18	3.313	5.363	236	3.872	5.600	702	4.109	
4.827	20	3.336	5.369	243	3.878	5.604	716	4.113	
4.857	23	3.366	5.376	251	3.885	5.608	729	4.117	
4.876	25	3.385	5.383	259	3.892	5.612	743	4.121	
4.892	27	3.401	5.390	267	3.899	5.616	757	4.125	
4.915	30	3.424	5.396	275	3.905	5.620	771	4.129	
4.936	33	3.445	5.403	284	3.912	5.624	785	4.133	
4.955	36	3.464	5.409	292	3.918	5.628	800	4.137	
4.972	39	3.481	5.415	300	3.924	5.632	814	4.141	
4.988	42	3.497	5.422	309	3.931	5.636	829	4.145	
5.003	45	3.512	5.428	318	3.937	5.640	845	4.149	
5.017	48	3.526	5.434	327	3.943	5.644	861	4.153	
5.035	52	3.544	5.440	336	3.949	5.648	877	4.157	
5.051	56	3.560	5.446	345	3.955	5.652	893	4.161	
5.062	59	3.571	5.451	354	3.960	5.656	909	4.165	
5.076	63	3.585	5.457	364	3.966	5.660	925	4.169	
5.090	67	3.599	5.462	373	3.971	5.663	941	4.172	
5.102	71	3.611	5.468	383	3.977	5.667	957	4.176	
5.114	75	3.623	5.473	392	3.982	5.671	973	4.180	
5.128	80	3.637	5.479	402	3.988	5.674	989	4.183	
5.139	84	3.648	5.484	412	3.993	5.678	1005	4.187	
5.151	89	3.660	5.489	422	3.998				
5.163	94	3.672	5.495	433	4.004				
5.172	98	3.681	5.500	443	4.009				
5.183	103	3.692	5.505	453	4.014				
5.193	108	3.702	5.510	464	4.019				
5.205	114	3.714	5.515	474	4.024				
5.214	119	3.723	5.520	486	4.029				

Check on Inverse's

DEPARTMENT OF COMMERCE  
U. S. COAST AND GEODETIC SURVEY  
FORM 27  
Ed. April, 1929

POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

$\alpha$	2	to 3	to 2	$\alpha$	3	to 2	to 3
$2^d \angle$		+		$3^d \angle$		&	
$\alpha$	2	187 31 07.2	to 1	$\alpha$	3	to 1	29 25.4
$\Delta \alpha$		+	90.7	$\Delta \alpha$			01 11.6
		180 00 00.0			180	00 00.0	
$\alpha'$	1	7 31 17.4	to 2	$\alpha'$	1	to 3	218 28 13.8 ✓

FIRST ANGLE OF TRIANGLE

$\phi$	40	47	18.249	2 N. Tower E. L.	$\lambda$	73	58	04.044	$\phi$	40	50	33.166	3 Highbridge	$\lambda$	73	55	58.826
$\Delta \phi$	+	01 30.231	30.231		$\Delta \lambda$	+		15.674	$\Delta \phi$	-	01 44.686	44.686		$\Delta \lambda$	+	01 49.544	49.544
$\phi'$	40	48	48.480	1 Dome Grantsford	$\phi'$	40	57	48.370	$\phi'$	40	48	48.480	1 Dome (Grantsford)	$\phi'$	40	57	48.370
$s$	3.448316				$\frac{1}{2}(\phi + \phi')$	40-48-03.4			$s$	3.615438				$\frac{1}{2}(\phi + \phi')$	40-49-40.8		
$\cos \alpha$	9.996250				Logarithms	9.996250			$\cos \alpha$	9.893602				Logarithms	9.794058		
B	8.510792				$s$	3.448316			B	8.510788				$s$	3.615438		
$h$	1.955358				$\sin \alpha$	9.116771			$h$	2.019828			1st term	$\sin \alpha$	9.794058		
$s^2$	6.8966				$\sin^2 \alpha$	8.509098			$s^2$	7.2309				$\sin^2 \alpha$	8.509098		
$\sin^2 \alpha$	8.2335				Sec $\phi'$	0.120995			$\sin^2 \alpha$	9.5881				Sec $\phi'$	0.120995		
C	1.34403				$\Delta \lambda$	1.195180		15.6740	C	1.3410				$\Delta \lambda$	2.039589		109.544
	6.4704				$\sin \frac{1}{2}(\phi + \phi')$	9.915201				8.1600			2d term	$\sin \frac{1}{2}(\phi + \phi')$	9.815439		
$h^2$	3.911				$-\Delta \alpha$	1.010381		10.24	$h^2$	4.040				$-\Delta \alpha$	1.855028		71.67
D	2.388								D	2.388							
	6.29				3d term	+.0002				6.428			3d term	+.0003			
					$-\Delta \phi$	90.731							$-\Delta \phi$	104.6863			

DEPARTMENT OF COMMERCE  
U. S. COAST AND GEODETIC SURVEY  
FORM 25  
Ed. Jan., 1920

## COMPUTATION OF TRIANGLES

State: \_\_\_\_\_

11-9121

NO.	STATION	OBSERVED ANGLE	CORR'N	SPHER'L ANGLE	SPHER'L EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3	Mt. Morris Park						3.448316 ✓
1	Flagpole ecc.	73 09 19.2 ✓					0.0190455
2	U.S. Tower						9.8184736
2	Eldorado Dome	41 10 33.9 ✓					9.9596030 ✓
3	(Grants Tomb)	(65 40 06.8)					3.285835 ✓
1-3					0.0		3.426969 ✓
1-2							
2-3	Mt. Morris Park						3.615438 ✓
1	Flagpole ecc.	70 26 25.6 ✓					0.0258136
2	Dome						9.9970968 ✓
2	(Grants Tomb)	(83 22 56.8)					9.6445841
3	Highbridge 1898	26 10 37.9 ✓					3.638348 ✓
1-3					0.0		3.285836 ✓
1-2							
2-3							
1							
2							
3							
1-3							
1-2							
2-3							
1							
2							
3							
1-3							
1-2							

Do not write in this margin

POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

FIRST ANGLE OF TRIANGLE															
°		'		"		°		'		"		°			
$\alpha$	2	to 3		187	31	07.2	$\alpha$	3	to 2		7	31	17.4		
$2^d \angle$		&		+ 41	10	35.8	$3^d \angle$		&		- 65	40	06.8		
$\alpha$	2	to 1		228	41	41.2	$\alpha$	3	to 1		301	51	10.6		
$\Delta \alpha$						18.0	$\Delta \alpha$						45.7		
				180	00	00.0					180	00	00.0		
$\alpha'$	1	to 2		48	42	37.2	$\alpha'$	1	to 3		121	51	56.3		
FIRST ANGLE OF TRIANGLE															
$\phi$	40	47	18.249	2.166	73	58	04.044	$\phi$	40	48	48.480	3.686	73	57	48.370
$\Delta \phi$			57.185			01	25.655	$\Delta \phi$			33.046			01	09.981
$\phi'$	40	48	15.434	1.143	73	56	38.389	$\phi'$	40	48	15.434	1.143	73	56	38.389
VALUES IN SECONDS															
$s$	3.426964								$s$	3.285835					
$\cos \alpha$	9.8195904								$\cos \alpha$	9.7224204					
B	8.507912								B	8.5107898					
h	1.7573461								h	1.5190452		33.0404			
$s^2$	6.8539								$s^2$	6.5716					
$\sin^2 \alpha$	9.7515								$\sin^2 \alpha$	9.8582					
C	1.3404								C	1.3408					
	7.9458									7.7706					
$h^2$	3.5197								$h^2$	3.0381					
D	2.3877								D	2.3877					
	5.9024									5.4258					
VALUES IN SECONDS															
1st term	57.1934								1st term	33.0404					
2d term	+0.0088								2d term	+0.0059					
3d term	+0.0001								3d term	+0.0001					
$-\Delta \phi$	57.1845								$-\Delta \phi$	33.0463					
VALUES IN SECONDS															
$\frac{1}{2}(\phi + \phi')$									$\frac{1}{2}(\phi + \phi')$						
$s$									$s$						
$\sin \alpha$									$\sin \alpha$						
$A'$									$A'$						
$\sec \phi'$									$\sec \phi'$						
$\Delta \lambda$									$\Delta \lambda$						
$\sin \frac{1}{2}(\phi + \phi')$									$\sin \frac{1}{2}(\phi + \phi')$						
$-\Delta \alpha$									$-\Delta \alpha$						
1st term	57.1934								1st term	33.0404					
2d term	+0.0088								2d term	+0.0059					
3d term	+0.0001								3d term	+0.0001					
$-\Delta \phi$	57.1845								$-\Delta \phi$	33.0463					

ON N.A. DATUM

POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

<u>St. Morris Pt</u>		<u>N. Tower</u>			
$\alpha$	2	to 3	to 2		
$2^d \angle$		&			
$\alpha$	2	to 1	to 1		
$\Delta \alpha$					
$\alpha'$	1	to 2	to 3		
				180	00 00.0
				182	30 17.2

FIRST ANGLE OF TRIANGLE

		° ' "		° ' "	
$\phi$	40 48	15.434	2	73 56	38.389
$\Delta \phi$		14.198	2		10.008
$\phi'$	40 48	15.286	1	73 56	38.397
Values in seconds					
Logarithms					
$s$	0.658965				
$\cos \alpha$	9.999585				
B	8.510790				
h	9.169340				
$s^2$	13.179				
$\sin^2 \alpha$	728.08				
C	1.3466				
$h^2$					
D	2.3877				
Logarithms					
$\frac{1}{2}(\phi + \phi')$					
$s$	0.658965				
$\sin \alpha$	8.690999				
A'	8.509098				
$\sec \phi'$	0.120935				
$\Delta \lambda$	7.929497				
$\sin \frac{1}{2}(\phi + \phi')$	9.815				
$-\Delta \alpha$	7.744				
Values in seconds					
1st term	0.1477				
2d term	+				
3d term	+				
$-\Delta \phi$					
Logarithms					
$\frac{1}{2}(\phi + \phi')$					
$s$					
$\sin \alpha$					
A'					
$\sec \phi'$					
$\Delta \lambda$					
$\sin \frac{1}{2}(\phi + \phi')$					
$-\Delta \alpha$					
Values in seconds					
1st term					
2d term	+				
3d term	+				
$-\Delta \phi$					

Above position on N.A. Datum

For N.A. 1927 Datum  
Apply Lat. -12.0m  
Long + 3.6m

N.A. 1927 Datum Position  
40° 48' - 459.5m  
73° 56' - 903.6m



# Plane coordinates on Lambert projection

State L. Island Station St Johns 1932 (NM)

$\phi = 40^{\circ} 48' 18.214''$   $\lambda = 73^{\circ} 57' 47.186''$

Tabular difference of R for 1" of  $\phi = 101.20267$

R (for min. of $\phi$ )		24 253 248.71	y' (for min. of $\phi$ )		209,296.59
Cor. for sec. of $\phi$		- 1 843.31	Cor. for sec. of $\phi$	+	1 843.31
R		24,251,405.40	y'		211,139.90
			y'' (= $2R \sin^2 \frac{\theta}{2}$ )	+	2.15
$\theta$ (for min. of $\lambda$ )		+ $0^{\circ} 01' 57.73478''$	y		211,142.05 ✓
Cor. for sec. of $\lambda$		- 30.86352			
$\theta$		+ 1 26.87126	$\frac{\theta}{2}$		$^{\circ} 43.4$
$\theta''$	For machine computation	86." 87126		For machine computation	
			log $\theta''$		1.93888
log $\theta''$		1.93887612	colog 2		9.69897000
S for $\theta$		4.68557485	S for $\frac{\theta}{2}$		4.68557
log sin $\theta$	sin $\theta$		log sin $\frac{\theta}{2}$	sin $\frac{\theta}{2}$	6.32342
log R		7.38473691		R sin $\frac{\theta}{2}$	
log x'		4.00918788	log sin <sup>2</sup> $\frac{\theta}{2}$	R sin <sup>2</sup> $\frac{\theta}{2}$	2.64684
x'	R sin $\theta$	+ 10,213.81	log R		7.38474
		2,000,000.00	log 2		0.30103000
x		2,010,213.81	log y''		0.33261

$$x = 2,000,000.00 + R \sin \theta$$

$$y = y' + 2R \sin^2 \frac{\theta}{2}$$

y' = the value of y on the central meridian for the latitude of the station

S = log of ratio for reducing arc expressed in seconds to sine

(see log tables)

R, y', and  $\theta$  are given in special tables

## Geodetic positions from Lambert coordinates

State \_\_\_\_\_ Station \_\_\_\_\_

x	2,014,000.00	$R_b + A$	24,462,545.30
C	2,000,000.00	y	210,000.00
$x' (= x - C)$	+ 14,000.00	$R_b + A - y$	24,252,545.30
	4.14612804		
	7.38475733		
tan $\theta$	6.76137071	R	17 7.38475741
$\theta$	{ 0° 01' 59.06847"		
	+ 119.06847"	y	210,000.00
$\frac{\theta}{\ell} (= \Delta \lambda)$	+ 182.0390	y''	- 4.04
		y'	209,995.96
$\lambda$ (central mer.)	74° 00' 00.0000		
$-\Delta \lambda$	- 2 02.0390	$\phi$ (by interpolation)	40° 48' 06.9106
$\lambda$	73 56 57.9610		426.3
	1310.86		(1424.5)
	(95.6)		1850.8
	1406.5		

Station \_\_\_\_\_

x	2,022,000.00	$R_b + A$	24,462,545.30
C	2,000,000.00	y	204,000.00
$x' (= x - C)$	22,000.00	$R_b + A - y$	24,258,545.30
	4.34242265		
	7.38486475		
tan $\theta$	6.95755793	R	17 7.38486493
$\theta$	{ 0° 02' 07.06102"		
	+ 187.06102"	y	204,000.00
$\frac{\theta}{\ell} (= \Delta \lambda)$	+ 285.9901	y''	- 9.94
		y'	203,990.02
$\lambda$ (central mer.)	74° 00' 00.0000		
$-\Delta \lambda$	- 04 45.9901	$\phi$ (by interpolation)	40° 47' 07.5647
$\lambda$	73 55 14.0099		466.7
	657.0		(1384.1)
	(749.2)		1850.8
	1406.8		

$$\tan \theta = \frac{x - C}{R_b + A - y}$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

$$\Delta \lambda = \frac{\theta}{\ell}$$

C is constant added to  $x'$  in computation  
of coordinates

$$\lambda = \lambda (\text{central mer.}) - \Delta \lambda$$

$R_b$  is map radius of lowest parallel

$$R = (R_b + A - y) \sec \theta$$

A is value of  $y'$  for  $R_b$ ; in most cases it is zero

$\phi$  is interpolated from table of  $y'$

# Geodetic positions from Lambert coordinates

State \_\_\_\_\_ Station \_\_\_\_\_

x	2,022,000.00	R <sub>b</sub> + A	24,462,545.30
C	2,000,000.00	y	216,000.00
x' (= x - C)	+ 22,000.00	R <sub>b</sub> + A - y	24,246,545.30
	4.3424 2268		
	7.3846 4987		
tan θ	6.9577 7281	R	17 7.3846 5005
θ	0° 03' 07.15379"		
	+ 187.15379	y	216,000.00
$\frac{\theta}{\ell}$ (= Δλ)	+ 286.1320	y''	- 9.98
		y'	215,990.02
λ (central mer.)	74° 00' 00.0000"		
- Δλ	- 0° 04' 46.1320"	φ (by interpolation)	40° 49' 06.1384"
λ	73° 55' 13.8680"		378.7
	650.0		(1472.1)
	(756.1)		1850.8
	1406.1		

Station \_\_\_\_\_

x		R <sub>b</sub> + A	
C		y	
x' (= x - C)		R <sub>b</sub> + A - y	
tan θ		R	
θ	° ' "		
	"	y	
$\frac{\theta}{\ell}$ (= Δλ)		y''	-
		y'	
λ (central mer.)	° ' "		
- Δλ		φ (by interpolation)	° ' "
λ			

$$\tan \theta = \frac{x - C}{R_b + A - y}$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

$$\Delta \lambda = \frac{\theta}{\ell}$$

C is constant added to x' in computation of coordinates

$$\lambda = \lambda (\text{central mer.}) - \Delta \lambda$$

R<sub>b</sub> is map radius of lowest parallel

$$R = (R_b + A - y) \sec \theta$$

A is value of y' for R<sub>b</sub>; in most cases it is zero

φ is interpolated from table of y'

State \_\_\_\_\_ Station \_\_\_\_\_

x	2,006,000.00	$R_b + A$	24,462,545.30
C	2,000,000.00	y	204,000.00
$x' (= x - C)$	+ 6,000.00	$R_b + A - y$	24,258,545.30
	3,778 15 125		
	7,384 86 475		
tan $\theta$	6393 28 650	R	log 7,384 86 475
$\theta$	{ 0 0 51.01677		
	+ 51.01677	y	204,000.00
$\frac{\theta}{\ell} (= \Delta \lambda)$	+ 77.9975	y''	- .74
		y'	203,999.26
$\lambda$ (central mer.)	74 00 00.0000		
$-\Delta \lambda$	- 01 17.9975	$\phi$ (by interpolation)	40 47 07.6500
$\lambda$	73 58 42.0025		472.0
	562.8		(1378.8)
	(844.0)		1850.8
	1406.8		

Station \_\_\_\_\_

x	2,006,000.00	$R_b + A$	24,462,545.30
C	2,000,000.00	y	204,000.00
$x' (= x - C)$	+ 6,000.00	$R_b + A - y$	24,258,545.30
	3,778 15 125		
	7,384 64 947		
tan $\theta$	6393 58 738	R	log 7,384 64 948
$\theta$	{ 0 0 51.04225		
	+ 51.04225	y	215,000.00
$\frac{\theta}{\ell} (= \Delta \lambda)$	+ 78.0365	y''	- .74
		y'	215,999.26
$\lambda$ (central mer.)	74 00 00.0000		
$-\Delta \lambda$	- 01 18.0365	$\phi$ (by interpolation)	40 49 06.2302
$\lambda$	73 58 41.9635		384.4
	560.8		(1466.4)
	(845.3)		1850.8
	1406.1		

$$\tan \theta = \frac{x - C}{R_b + A - y}$$

$$\Delta \lambda = \frac{\theta}{\ell}$$

$$\lambda = \lambda (\text{central mer.}) - \Delta \lambda$$

$$R = (R_b + A - y) \sec \theta$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

C is constant added to  $x'$  in computation  
of coordinates

$R_b$  is map radius of lowest parallel

A is value of  $y'$  for  $R_b$ ; in most cases it is zero

$\phi$  is interpolated from table of  $y'$

Report for Supplemental T 5452  
3/8/39

The corrections shown in red on T 5452 <sup>Supplemental</sup> were plotted in this office from single lens air photographs without field inspection.

Field inspection is now in progress and any additional corrections resulting from the field inspection will be added in another color as soon as the field inspection data is available.

Photographs Single lens - 7" x 9" - scale 1:10,000  
negatives on file in this office. Photographs taken by Naval Air Station Unit Washington, D.C. early in Feb. 1939, exact date not furnished.

Pilot Details in red plotted by T.A. McGann.  
Radial plot on a separate projection from radio prints scale 1:5,000. Corrections transferred to the supplemental by T.A. McGann.

Hydrographic Survey Shore line on the hydrographic survey of this area is from T 5453 prior to the above corrections. The corrections on T 5453 Supplemental have not been applied to the Hydrographic sheet.

B.G. Jones 3/9/39.

Additional details in blue added 4/2/39 after checking with the field inspection. Field inspection notes are shown on the photographs and on C 158 (air photo unit files).