

5450

5450

Form 504 Rev. Dec. 1933	
DEPARTMENT OF COMMERCE U.S. COAST AND GEODETIC SURVEY R. S. PATTON, DIRECTOR	
DESCRIPTIVE REPORT	
Air Photo Topographic Hydrographic	Sheet No. T-5450
State <u>New Jersey</u>	
LOCALITY	
Hudson River	
Hoboken and Jersey City	
Date of Photos ¹⁹³⁵ 1935 Nov. 1934	
CHIEF OF PARTY	
J.C. Partington	

Applied to Chart ~~369~~ 746 - May 1937 - R.M.Z.
Applied to Chart 369 - April 1939 R.M.Z.

- 1 -
DEPARTMENT OF COMMERCE
U.S. COAST AND GEODETIC SURVEY

REG. NO.

TOPOGRAPHIC TITLE SHEET

T5450

The Topographic Sheet should be accompanied by this form, filled in as completely as possible, when the sheet is forwarded to the Office.

Field No. 82

REGISTER NO. T-5450

State New Jersey

General locality Hudson River

Locality Hoboken and Jersey City

Scale 1: 5000 Date of survey photographs Nov. 25, 1934

Date of Compilation July 20, 1936

Vessel Air Photo Compilation Party # 25

Reviewed and recommended for approval
Chief of party J.C. Partington

Surveyed by See STATISTICS SHEET, page 2 of this report.

Inked by J.K. Batchellor and R.H. Young

Heights in feet above - - - to ground to tops of trees

Contour, Approximate contour, Form line interval - - - feet

Instructions dated March 14, 1934

Remarks: Compiled on a scale of 1: 5000 and printed by

photo-lithography.

STATISTICS

on

AIR PHOTO TOPOGRAPHIC SHEET, FIELD NO. 82 ; REGISTER NO. T-5450

PHOTOGRAPH NO.	DATE	TIME	TIDE			
			High		Low	
			Time	Ht.	Time	Ht.
V 77 to 85(870 M-8)	Nov.25,1934	10:50 AM	10:54 AM	4.3	4:52 Am	0.3
			11:47 PM	3.5	5:19 PM	0.0

	By	Date
SCALE FACTOR (1.000)	R.C.Bolstad	(Previously determined)
PROJECTION	R.H.Peckworth	3-8-35
PROJECTION CHECKED	D.B.Bennett	3-8-35
CONTROL PLOTTED	R.H.Peckworth	3-11-35
CONTROL CHECKED	D.B.Bennett	3-12-35
SMOOTH RADIAL LINE PLOT	R.H.Peckworth	
RADIAL LINE PLOT CHECKED	R.C.Bolstad	March 1936
DETAIL INKED	J.K.Batchellor R.H.Young	5-11-36 to 6-11-36 6-26-36 to 7-15-36
PRELIMINARY REVIEW OF SHEET	<i>J.C. Partington</i> J.C.Partington	7-16-36 to 7-20-36
AREA OF DETAIL INKED (land area)	<u>4.1</u>	Square Statute Miles
AREA OF DETAIL INKED (shoals)	<u>0.0</u>	Square Statute Miles
LENGTH OF SHORELINE (more than 200 M. from opposite shore)	<u>26.7</u>	Stat. Miles
LENGTH OF SHORELINE (rivers & sloughs less than 200 M.wide)	<u>2.9</u>	Stat.Miles
LENGTH OF STREETS, ROADS, RAILROADS, TRAILS	<u>383.1</u>	Stat. Miles
GENERAL LOCATION	<u>New Jersey</u>	
LOCATION	<u>Hoboken and Jersey City</u>	
DATUM	<u>North American 1927</u>	

STATION	<u>Castle Stevens, 1875</u> <u>r' 33 (N.J.)</u>	Latitude	40° 44 ' 41.152"	1269.4 m.
		Longitude	74 01 28.040	657.9 m.

(Adjusted computations)

Long Island System of Plane Coordinates.
 Castle Stevens, 1875. $x = 1,993,223.32$ ft. ✓
 $y = 189,173.71$ ft.

New Jersey System of Plane Coordinates
 Castle Stevens, 1875. $x = 2,177,954.74$ ft. ✓
 $y = 696,903.49$ ft.

COMPILER'S REPORT

for

AIR PHOTO TOPOGRAPHIC SHEET, FIELD NO. 82; REGISTER NO. T-5450

GENERAL INFORMATION.

The Air-photo Field Inspection Report for New Jersey, Part 1, Hudson River, George Washington Bridge to Bedloes Island, attached to the descriptive report for compilation T-5448, furnished the necessary information for the compilation of this sheet.

The sheet has been compiled from single lens photographs taken on Nov. 25, 1934 at the time of high water. The photographs were taken by the Army Air Corps at Mitchell Field, Long Island, N.Y. with a special camera known as the "K-7C" by the Army, but called the "K-7A" by the Fairchild Camera Corporation. This camera was recently developed by the Fairchild Camera Corporation, 62-10 Woodside Ave., Woodside, New York City. The Army plane was piloted by Lieut. Cullen at an altitude very close to 15,000 feet; the photographer was Sergeant Cates. A 24 inch cone (focal length 24") was used which placed the original negatives on a scale of 1: 7500. Contact prints were furnished the field party for inspecting purposes and the original negatives were used to enlarge a set of office prints to a scale of 1: 5000. The enlarging was done in the Washington office of the U.S. Coast and Geodetic Survey. The office prints were furnished the field party and were used in the compilation of this sheet.

CONTROL.

(a) Sources.

Control for the compilation of this sheet was obtained from the following sources:

1. Triangulation 1930-33 by R.W. Woodworth. As the final office adjusted positions on N.A. 1927 datum were not available except for main scheme stations, the field positions on N.A. datum were used after applying a correction determined by a comparison of common stations for the area. The following corrections were applied to the N.A. datum to convert to the N.A. 1927 datum:
Latitude minus 12.0 meters
Longitude plus 3.2 meters
2. U.S. Engineers stations described on Form 524 submitted with this report.
3. Stork Line Tank, a 3 point theodolite fix attached to this report and described on Form 524.

(b) Errors.

No error was discovered in the position of any of the triangulation stations and only one U.S. Engineers station was found to be incorrect in position, namely Castle Stevens (U.S.E.)

The radial line plot position of "Castle Stevens" (U.S.E.) does not agree with the U.S.E. coordinate position. (See Form 524 for the station). The coordinate position of triangulation station "Castle Stevens" agrees with the geographic position. The station "Castle Stevens" (U.S.E.) is on the same structure and lies 17.77 feet S.E. of the triangulation station. The distance 17.77 feet was verified by R.W. Woodworth in 1932 on recovery card No. 349 R.

(b) Errors. (cont.)

The U.S.E coordinates do not conform to this measurement. The spotting on the photos was verified and conforms to field inspection data; it also agrees with sketches by the U.S. Engineers and by R.W. Woodworth.

The correct position of this station is shown on this ~~sheet~~ ^{compilation} by a $2\frac{1}{2}$ m m circle. This position lies about 25 feet south of the U.S.E. coordinate position.

COMPILATION.

(a) Method.

The usual radial line method of plotting was used in the ~~compilation of this sheet~~ ^{compilation}.

The U.S.E. stations described on Form 524 attached to this report were used as supplementary control for the radial line plot. They were plotted on an aluminum sheet from their coordinate positions and transferred to the celluloid sheet by fitting coordinate positions of triangulation stations on the aluminum sheet to their corresponding geographical position on the celluloid sheet. The transferred position of the U.S.E. stations was not accepted unless it adhered strictly to the radial line plot, as there is sufficient triangulation control to independently establish the plot.

(b) Adjustments of Plot.

No unusual adjustments of the plot were effected. Some adjustment was necessary along the western boundary of this ~~sheet~~ ^{compilation} where a junction is made with Compilation No. T-5277 (1: 10000 scale). Due to the weak angle of intersection in this area the radials give a somewhat weak determination in an east and west direction with a strong determination in a north and south direction. The 1: 10000 scale plot was used to strengthen the positions of these radial points and to obtain an agreement at the junction of the two ~~sheets~~ ^{compilations}.

(c) Interpretation.

No great difficulty was experienced in interpreting the photographic detail for this compilation.

The usual symbols were used as approved by the Board of Surveys and Maps (1932). All of the buildings along the waterfront have been shown and some of the more important buildings inland. Appropriate notes on the overlay sheet have been used to designate features which are not self-evident.

In areas where elevated tracks obscure the curb line of streets the curb line has been omitted to avoid a confusion of lines on the sheet.

(d) Information from Other Sources.

Railroad blue-prints showing the track layouts were used in interpreting detail in regard to the railroads shown on this

* A more reasonable estimate of the error of position is from 0.3 to 0.5 mm for intersected points and from 0.3 to 0.8 mm for points between intersections.

Lam.
Aug. 25, 1936.

(d) Information from Other Sources. (cont.)

sheet. The blue-prints were also used for obtaining names of streets, etc.

(c) Names.

All geographical names shown on this compilation have been listed on the special forms in the back of this report. For street names see the blue-prints enclosed with this sheet, and the Port Authority Maps.

COMPARISON WITH OTHER SURVEYS.

No other surveys of this area are on hand in this party and no comparison with other surveys has been made.

No direct comparison of this compilation with the charts by means of contact negatives has been made since the negatives were not available at the time of completion of this ~~sheet~~ ^{Compilation.}

JUNCTIONS.

This ~~sheet~~ ^{Compilation} joins compilation T-5449 (1: 5000 scale) on the north and compilation T-5470 (1: 5000 scale) on the south. These junctions are in agreement.

This ~~sheet~~ ^{Compilation} joins compilation T-5277 (1: 10000 scale) on the west and the junction between the two ~~sheets~~ ^{Compilations} is in agreement.

LANDMARKS.

The list of landmarks for the area covered by this compilation were previously submitted, Feb. 22, 1933, By R.W. Woodworth. *Chart Letter 176 (1933)*

LIST OF RECOVERABLE STATIONS.

Eight ^{descriptive} cards Form 524 accompany this report. Seven of these stations were located by the radial line plot and the eighth station, "Stork Line Tank", was located by a 3 point theodolite fix.

BRIDGES.

Only one bridge ^{over a water area} is shown on this compilation, located at the west end of the Morris and Essex Canal. It is of no importance to navigation.

RECOMMENDATIONS FOR FURTHER SURVEYS. *(see opp. page.)*

This compilation of ~~this sheet~~ is believed to have a probable error of not over 1 meter in position for well defined waterfront detail of importance for charting, not over 2 meters for other waterfront detail, and not to exceed 3 meters for inland detail.

This compilation is believed to be complete in all detail of importance for charting and no additional surveys are recommended.

Submitted by

J.C. Partington
J.C. Partington
Chief of Party # 25

	Remarks	Decisions
1		
2		
3	* Not Shown * <i>not shown</i>	
4	* Not Shown <i>not shown</i>	
5		
6	* Not Shown * <i>not shown</i>	
7		
8	* Not Shown	
9		
10	* Not Shown	
11	* Not Shown	
12	* Not Shown ** Shown as "Holland Vehicular Tunnel".	
13	* Hudson and Manhattan R.R.	
14	** Shown as "P. R. R. Station L. V. R. R."	
15	* Not Shown	
16	* Not Shown	
17	* Not Shown * ** Shown as "Morris Canal Basin".	
18	* Not Shown	
19	* Not Shown	
20	** Not Shown	
21		
22		
23		
24	* not shown	
25	* not shown ** Shown as " Church Sq. Pk."	
26		
27		
M 234		

GEOGRAPHIC NAMES
Survey No. T-5450
Air Photo Compilation

Name on Survey	Chart 145									
	A	B	C	D	E	F	G	H	K	
Hoboken	x			x	x	/	/		1	
Jersey City	x			x	x	/	/		2	
Castle Point	x			*	*				3	
Erie R. R.	x			x	*	Rt. Guide			4	
Washington St.	x			x	x				5	
Hoboken Manufacturers RR	x			x	*				6	
Newark St.	x			x	x				7	
D.L. and W. R.R. Yards	x			x	*				8	
Henderson St.	x			x	x				9	
Erie R. R. Yards	x			x	*				10	
Hudson and Manhattan Tubes *				x	*				11	
Holland Tunnel /	x			x	*				12	
Pennsylvania Railroad	x			x	*		/		13	
Pennsylvania R.R. Station	x			*	x	Rt. Guide			14	
Essex St.	x			*	*				15	
Morris Basin	x			*	*				16	
Morris and Essex Canal	x			*	**				17	
Lehigh Valley Railroad	x			x	*	Rt. Guide	/		18	
C.R.R. of N.J.	x			x	*	Rt. Guide	/		19	
C.R.R. of N.J. Station	x			x	x				20	
Weehawken Cove	x								21	
Elysian Field	x								22	
Canal Basin	x								23	
Hudson Square	x			*	*				24	
Church Square	*			**	*				25	
									26	
									27	

Authority N.Y.C. R.R. Termin. Map
Estimate and Apportionment
O. Guide or Map
Rand McNally Atlas
U. S. Light List

From local information
Map of U. S. quadrangle
On U. S. quadrangle
On previous survey
On Chart No.

Names underlined in red approved by [signature] on 8/15/36

M 234

Names underlined in red approved
by C.R.D. on 8/17/36

Station: Ken

State: Maryland

Chief of party: C. V. H.

Date: 1917

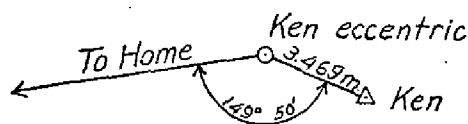
Computed by: O. P. S.

Observer: C. V. H.

Instrument: No. 168

Checked by: W. F. R.

OBSERVED STATION	Observed direction	Eccentric reduction	Sea level reduction	Corrected direction with zero initial	Adjusted direction
Chevy	0 00 00.00	- 7.31	"	0 00 00.00	"
Tank west of Δ Dulce	29 03 37.0	-1 09.8	"	29 02 34.5	"
Ken (center), 3.469 meters	176 42				
Forest Glen standpipe	313 24 53.0	+3 01.2		313 28 01.5	
Home	326 31 30.21	+ 31.93		326 32 09.45	
Bureau of Standards, wireless pole	352 17 20.8	+ 5.7		352 17 33.8	
Reno	357 28 48.63	- 1.16		357 28 54.78	
Reference mark, 16.32 m.	358 31 20				



This form, with the first three and fifth columns properly filled out and checked, must be furnished by field parties. *To be acceptable it must contain every direction observed at the station.*

It should be used for observations with both repeating and direction theodolites.

The directions at only one station should be placed on a page.

If a repeating theodolite is used, do not abstract the angles in tertiary triangulation. The local adjustment corrections (to close horizon only) are to be written in the Horizontal Angle Record, and the List of Directions is to be made from that record directly.

Choose as an initial for Form 24A some station involved in the local adjustment, and preferably one which has been used as an initial for a round of directions on objects not in the main scheme. Use but one initial at a station. Call the direction of the initial $0^{\circ} 00' 00."$ 00, and by applying the corrected angles to this, fill in opposite each station its direction reckoned *clockwise* around the whole circumference regardless of the direction of graduation of the instrument. The clockwise reckoning is necessary for uniformity and to make the directions comparable with azimuths.

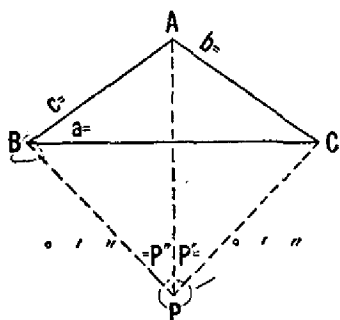
If a station has been occupied eccentrically, reduce to the center and enter in this form, in ink, the resulting corrections to the observed directions in the column provided for them. If an eccentric reduction is necessary, but not made in the field, leave the column blank. If the station was occupied centrally, and no eccentric reduction is required, put dashes in the column to show that no corrections are necessary.

Directions in the main scheme should be entered to hundredths of seconds in first-order triangulation; otherwise to tenths only. Points observed upon but once, direct and reverse, should be carried to tenths in first-order and second-order triangulation, and to even seconds only in third-order triangulation. In general, but two uncertain figures should be given.

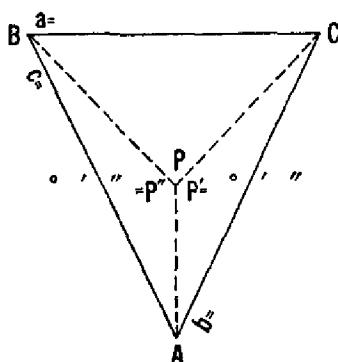
It is recommended that the following simple plan of observing be used with a repeating instrument: Measure each single angle in the scheme at each station and the outside angle necessary to close the horizon. *Measure no sum angles.* Follow each measurement of every angle immediately by a measurement of its supplement. Six repetitions are to constitute a measurement. The local adjustment will consist simply of the distribution of the error of closure of the horizon.

COMPUTATION OF THREE-POINT PROBLEM

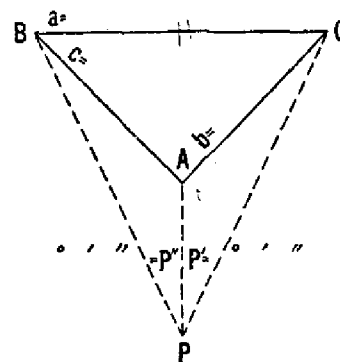
Case 1



Case 2



Case 3



Cases 1 and 2

P'	65	39	32.5✓
P''	73	49	22.5✓
A	109	20	22.8✓

Sum	248	49	178✓
1/2 Sum	124	24	38.9✓

$$S = 180^\circ - \frac{1}{2} \text{sum} = 55 \quad 35 \quad 21.1 \checkmark$$

Log c	=	3.4028640✓
Log sin P'	=	9.9595103✓
Colog b	=	6.6721590✓
Colog sin P''	=	0.0175455✓

$$\text{Sum} = \log \tan Z = 0.0521388 \checkmark$$

Z	=	48	25	51.823✓
Z + 45°	=	93	25	51.823✓

Log cot (Z + 45°)	=	8.77782547✓
Log tan S	=	0.1643150✓

$$\text{Sum} = \log \tan \epsilon = 8.94214047 \quad (\text{sign } -)$$

ϵ	5	00	07.8✓
S	55	35	21.1✓

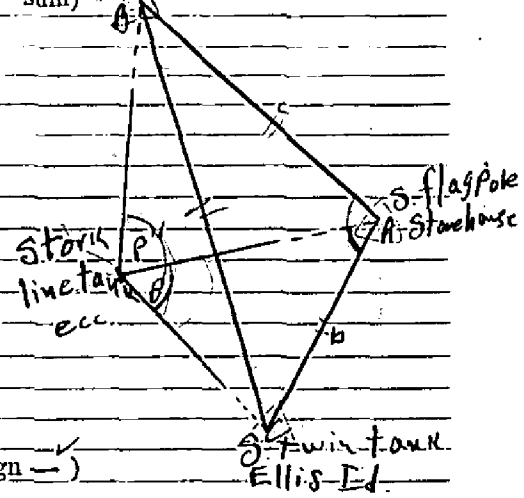
(Tan ϵ +)

S + ϵ = angle ABP	50	35	13.3✓
S - ϵ = angle ACP	60	35	28.9✓

Case 3

P'	
P''	
Sum	
A	

$$A - \text{sum} = \text{Dome (Counthouse)}$$



(Tan ϵ -)

S - ϵ = angle ABP			
S + ϵ = angle ACP			

BPA	APC	PCB
ABP	PCA	CBP
PAB	CAP	BPC

(For explanation of this form see Special Publication No. 138, pages 191 and 192, or Special Publication No. 145, pages 98-100)

INVERSE POSITION COMPUTATION

N.A. Datum

$$s_1 \sin \left(\alpha + \frac{\Delta \alpha}{2} \right) = \frac{\Delta \lambda_1 \cos \phi_m}{A_m}$$

$$s_1 \cos \left(\alpha + \frac{\Delta \alpha}{2} \right) = \frac{-\Delta \phi_1 \cos \frac{\Delta \lambda}{2}}{B_m}$$

$$-\Delta \alpha = \Delta \lambda \sin \phi_m \sec \frac{\Delta \phi}{2} + F(\Delta \lambda)^2$$

in which $\log \Delta \lambda_1 = \log (\lambda' - \lambda)$ - correction for arc to sin*; $\log \Delta \phi_1 = \log (\phi' - \phi)$ - correction for arc to sin*; and $\log s = \log s_1 +$ correction for arc to sin*.

		NAME OF STATION	
1. ϕ	40 43 06.30	S. flag Pole (Storehouse)	74 02 00.59
2. ϕ'	40 43 53.83	Dome (Court House)	74 03 28.37
$\Delta \phi (= \phi' - \phi)$	+ 47.53		+ 1 28.78
$\frac{\Delta \phi}{2}$	23.765		
$\phi_m (= \phi + \frac{\Delta \phi}{2})$	40 43 30.065		
$\Delta \phi$ (secs.)	+ 47.53		+ 87.78
$\log \Delta \phi$	1.676 9678		1.943 3956
cor. arc - sin	-		-
$\log \Delta \phi_1$	1.676 9678		1.943 3956
$\log \cos \frac{\Delta \lambda}{2}$			9.879 5830
$\text{colog } B_m$	1.489 2034		1.490 9000
$\log \left[s_1 \cos \left(\alpha + \frac{\Delta \alpha}{2} \right) \right]$	3.166 1712 -	(opposite in sign to $\Delta \phi$)	$\log \left[s_1 \sin \left(\alpha + \frac{\Delta \alpha}{2} \right) \right]$ 3.313 8786 +
			$\log \left[s_1 \cos \left(\alpha + \frac{\Delta \alpha}{2} \right) \right]$ 3.166 1712 -
$\log \Delta \lambda$	1.943 3956	3 $\log \Delta \lambda$	$\log \tan \left(\alpha + \frac{\Delta \alpha}{2} \right)$ 0.147 7074
$\log \sin \phi_m$	9.814 5334	$\log F$	$\frac{-\Delta \alpha}{\alpha + \frac{\Delta \alpha}{2}}$ 2.100 8
$\log \sec \frac{\Delta \phi}{2}$		$\log b$	57 35 38.92
$\log a$	1.757 9290		$\log \sin \left(\alpha + \frac{\Delta \alpha}{2} \right)$ 9.911 0144
a			$\log \cos \left(\alpha + \frac{\Delta \alpha}{2} \right)$ 9.763 3071
b			$\log s_1$ 3.402 8642
$-\Delta \alpha$ (secs.)	+ 57.27		cor. arc - sin +
$\frac{\Delta \alpha}{2}$	28.64		$\log s$ 3.402 864
$\alpha + \frac{\Delta \alpha}{2}$	125 26 21.08		
α (1 to 2)	125 26 49.7		
$\Delta \alpha$	- 57.3		
	180		
α' (2 to 1)	305 25 52.4		

* Use the table on the back of this form for correction of arc to sin.

NOTE.—For $\log s$ up to 4.52 and for $\Delta \phi$ or $\Delta \lambda$ (or both) up to 10', omit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underscored, if using logarithms to 6 decimal places.

Table of arc-sin corrections for inverse position computations

$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	
4.177	1	2.686	5.223	124	3.732	5.525	497	4.034	
4.327	2	2.836	5.234	130	3.743	5.530	508	4.039	
4.415	3	2.924	5.243	136	3.752	5.534	519	4.043	
4.478	4	2.987	5.253	142	3.762	5.539	530	4.048	
4.526	5	3.035	5.260	147	3.769	5.543	541	4.052	
4.566	6	3.075	5.269	153	3.778	5.548	553	4.057	
4.599	7	3.108	5.279	160	3.788	5.553	565	4.062	
4.628	8	3.137	5.287	166	3.796	5.557	577	4.066	
4.654	9	3.163	5.294	172	3.803	5.561	588	4.070	
4.677	10	3.186	5.303	179	3.812	5.566	600	4.075	
4.697	11	3.206	5.311	186	3.820	5.570	613	4.079	
4.716	12	3.225	5.318	192	3.827	5.575	625	4.084	
4.734	13	3.243	5.326	199	3.835	5.579	637	4.088	
4.750	14	3.259	5.334	206	3.843	5.583	650	4.092	
4.765	15	3.274	5.341	213	3.850	5.587	663	4.096	
4.779	16	3.288	5.349	221	3.858	5.591	674	4.100	
4.792	17	3.301	5.356	228	3.865	5.595	687	4.104	
4.804	18	3.313	5.363	236	3.872	5.600	702	4.109	
4.827	20	3.336	5.369	243	3.878	5.604	716	4.113	
4.857	23	3.366	5.376	251	3.885	5.608	729	4.117	
4.876	25	3.385	5.383	259	3.892	5.612	743	4.121	
4.892	27	3.401	5.390	267	3.899	5.616	757	4.125	
4.915	30	3.424	5.396	275	3.905	5.620	771	4.129	
4.936	33	3.445	5.403	284	3.912	5.624	785	4.133	
4.955	36	3.464	5.409	292	3.918	5.628	800	4.137	
4.972	39	3.481	5.415	300	3.924	5.632	814	4.141	
4.988	42	3.497	5.422	309	3.931	5.636	829	4.145	
5.003	45	3.512	5.428	318	3.937	5.640	845	4.149	
5.017	48	3.526	5.434	327	3.943	5.644	861	4.153	
5.035	52	3.544	5.440	336	3.949	5.648	877	4.157	
5.051	56	3.560	5.446	345	3.955	5.652	893	4.161	
5.062	59	3.571	5.451	354	3.960	5.656	909	4.165	
5.076	63	3.585	5.457	364	3.966	5.660	925	4.169	
5.090	67	3.599	5.462	373	3.971	5.663	941	4.172	
5.102	71	3.611	5.468	383	3.977	5.667	957	4.176	
5.114	75	3.623	5.473	392	3.982	5.671	973	4.180	
5.128	80	3.637	5.479	402	3.988	5.674	989	4.183	
5.139	84	3.648	5.484	412	3.993	5.678	1005	4.187	
5.151	89	3.660	5.489	422	3.998				
5.163	94	3.672	5.495	433	4.004				
5.172	98	3.681	5.500	443	4.009				
5.183	103	3.692	5.505	453	4.014				
5.193	108	3.702	5.510	464	4.019				
5.205	114	3.714	5.515	474	4.024				
5.214	119	3.723	5.520	486	4.029				

INVERSE POSITION COMPUTATION

N.A. Datum

$$s_1 \sin \left(\alpha + \frac{\Delta \alpha}{2} \right) = \frac{\Delta \lambda_1 \cos \phi_m}{\Delta m}$$

$$s_1 \cos \left(\alpha + \frac{\Delta \alpha}{2} \right) = \frac{-\Delta \phi_1 \cos \frac{\Delta \lambda}{2}}{B_m}$$

$$-\Delta \alpha = \Delta \lambda \sin \phi_m \sec \frac{\Delta \phi}{2} + F(\Delta \lambda)^2$$

in which $\log \Delta \lambda_1 = \log (\lambda' - \lambda)$ - correction for arc to sin*; $\log \Delta \phi_1 = \log (\phi' - \phi)$ - correction for arc to sin*; and $\log s = \log s_1 +$ correction for arc to sin*.

NAME OF STATION	
1. ϕ	40° 43' 06.30" S flag pole (storehouse)
2. ϕ'	40° 42' 00.04" S twin tank (Ellis Idl)
$\Delta \phi (= \phi' - \phi)$	- 1 06.26
$\frac{\Delta \phi}{2}$	- 33.13
$\phi_m (= \phi + \frac{\Delta \phi}{2})$	40-42-33.17
$\Delta \phi$ (secs.)	- 66.26
$\Delta \lambda (= \lambda' - \lambda)$	74° 02' 00.59" ✓
$\frac{\Delta \lambda}{2}$	74° 02' 25.73" ✓
$\Delta \lambda$ (secs.)	+ 0 25.14
$\log \Delta \phi$	1.8212514
cor. arc-sin	-
$\log \Delta \phi_1$	1.8212514
$\log \cos \frac{\Delta \lambda}{2}$	1.4892022
$\text{colog } B_m$	3.3104536 + (opposite in sign to $\Delta \phi$)
$\log \left\{ s_1 \cos \left(\alpha + \frac{\Delta \alpha}{2} \right) \right\}$	3.3104536 +
$\log \Delta \lambda$	1.4003653
$\log \sin \phi_m$	9.814395
$\log \sec \frac{\Delta \phi}{2}$	1.214760
$\log a$	1.214760
a	
b	
$-\Delta \alpha$ (secs.)	+ 16.40
$\frac{\Delta \alpha}{2}$	+ 8.20
$\alpha + \frac{\Delta \alpha}{2}$	16 06 18.74
α (1 to 2)	16 06 26.9
$\Delta \alpha$	16.4
180	
α' (2 to 1)	196 06 10.5
$\log \Delta \lambda$	1.4003653
cor. arc-sin	-
$\log \Delta \lambda_1$	1.4003653
$\log \cos \phi_m$	9.8796861
$\text{colog } A_m$	1.4908995
$\log \left\{ s_1 \sin \left(\alpha + \frac{\Delta \alpha}{2} \right) \right\}$	2.7709509 +
$\log \left\{ s_1 \cos \left(\alpha + \frac{\Delta \alpha}{2} \right) \right\}$	3.3104536 +
$\log \tan \left(\alpha + \frac{\Delta \alpha}{2} \right)$	9.4604973
$\alpha + \frac{\Delta \alpha}{2}$	16 06 18.74
$\log \sin \left(\alpha + \frac{\Delta \alpha}{2} \right)$	9.4431095
$\log \cos \left(\alpha + \frac{\Delta \alpha}{2} \right)$	9.9826122
$\log s_1$	3.3278414
cor. arc-sin	+
$\log s$	3.327841

* Use the table on the back of this form for correction of arc to sin.

NOTE.—For $\log s$ up to 4.52 and for $\Delta \phi$ or $\Delta \lambda$ (or both) up to 10', omit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underscored, if using logarithms to 6 decimal places.

Table of arc-sin corrections for inverse position computations

$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	$\log s_1$	Arc-sin correction in units of seventh decimal of logarithms	$\log \Delta\phi$ or $\log \Delta\lambda$	
4.177	1	2.686	5.223	124	3.732	5.525	497	4.034	
4.327	2	2.836	5.234	130	3.743	5.530	508	4.039	
4.415	3	2.924	5.243	136	3.752	5.534	519	4.043	
4.478	4	2.987	5.253	142	3.762	5.539	530	4.048	
4.526	5	3.035	5.260	147	3.769	5.543	541	4.052	
4.566	6	3.075	5.269	153	3.778	5.548	553	4.057	
4.599	7	3.108	5.279	160	3.788	5.553	565	4.062	
4.628	8	3.137	5.287	166	3.796	5.557	577	4.066	
4.654	9	3.163	5.294	172	3.803	5.561	588	4.070	
4.677	10	3.186	5.303	179	3.812	5.566	600	4.075	
4.697	11	3.206	5.311	186	3.820	5.570	613	4.079	
4.716	12	3.225	5.318	192	3.827	5.575	625	4.084	
4.734	13	3.243	5.326	199	3.835	5.579	637	4.088	
4.750	14	3.259	5.334	206	3.843	5.583	650	4.092	
4.765	15	3.274	5.341	213	3.850	5.587	663	4.096	
4.779	16	3.288	5.349	221	3.858	5.591	674	4.100	
4.792	17	3.301	5.356	228	3.865	5.595	687	4.104	
4.804	18	3.313	5.363	236	3.872	5.600	702	4.109	
4.827	20	3.336	5.369	243	3.878	5.604	716	4.113	
4.857	23	3.366	5.376	251	3.885	5.608	729	4.117	
4.876	25	3.385	5.383	259	3.892	5.612	743	4.121	
4.892	27	3.401	5.390	267	3.899	5.616	757	4.125	
4.915	30	3.424	5.396	275	3.905	5.620	771	4.129	
4.936	33	3.445	5.403	284	3.912	5.624	785	4.133	
4.955	36	3.464	5.409	292	3.918	5.628	800	4.137	
4.972	39	3.481	5.415	300	3.924	5.632	814	4.141	
4.988	42	3.497	5.422	309	3.931	5.636	829	4.145	
5.003	45	3.512	5.428	318	3.937	5.640	845	4.149	
5.017	48	3.526	5.434	327	3.943	5.644	861	4.153	
5.035	52	3.544	5.440	336	3.949	5.648	877	4.157	
5.051	56	3.560	5.446	345	3.955	5.652	893	4.161	
5.062	59	3.571	5.451	354	3.960	5.656	909	4.165	
5.076	63	3.585	5.457	364	3.966	5.660	925	4.169	
5.090	67	3.599	5.462	373	3.971	5.663	941	4.172	
5.102	71	3.611	5.468	383	3.977	5.667	957	4.176	
5.114	75	3.623	5.473	392	3.982	5.671	973	4.180	
5.128	80	3.637	5.479	402	3.988	5.674	989	4.183	
5.139	84	3.648	5.484	412	3.993	5.678	1005	4.187	
5.151	89	3.660	5.489	422	3.998				
5.163	94	3.672	5.495	433	4.004				
5.172	98	3.681	5.500	443	4.009				
5.183	103	3.692	5.505	453	4.014				
5.193	108	3.702	5.510	464	4.019				
5.205	114	3.714	5.515	474	4.024				
5.214	119	3.723	5.520	486	4.029				

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 25
Ed. Jan., 1929

COMPUTATION OF TRIANGLES

State: _____

11-9121

NO.	STATION	OBSERVED ANGLE	CORR'N	SPHER'L ANGLE	SPHER'L EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3	stork line						3.402864✓
1	tank ecc.	73 49 22.5✓					0.0175455
2	Dome Cowho.	50 35 13.3✓					9.8879490
3	S. flagpole st. no.	(55 35 24.2)✓					9.9164621
1-3							3.308359✓
1-2							3.336872
2-3	stork line						3.327841✓
1	tank ecc.	65 39 32.5✓					0.0404297
2	S. flagpole st. no.	(53 44 58.6)✓					9.9065724
3	st. twin tank Id.	60 35 28.9✓					9.9400879
1-3							3.274843
1-2							3.308359✓
2-3							
1							
2							
3							
1-3							
1-2							
2-3							
1							
2							
3							
1-3							
1-2							

Do not write in this margin

POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

		° ' "		° ' "		° ' "		° ' "	
α	2	to 3	to 2	α	3	to 2	to 3	α	3
$2d\angle$		&	&	$3d\angle$					
α	2	to 1	to 1	α	3	to 1	to 1		
$\Delta\alpha$				$\Delta\alpha$					
α'	1	to 2	to 2	α'	1	to 3	to 3		
FIRST ANGLE OF TRIANGLE									
ϕ	40 43 06.300	25. flagpole	74 02 00.590	ϕ	40 42 00.040	25. twin tower	74 02 25.730		
$\Delta\phi$		27.716	+	$\Delta\phi$		43.544	+		
ϕ'	40 42 43.584	storm line 1 tower ecc.	74 03 21.945	ϕ'	40 42 43.584	storm line 1 tower ecc.	74 03 21.945		
VALUES IN SECONDS									
s	3.308359		40-42-54.9	s	3.274843		40-42-21.8		
$\cos\alpha$	9.537016			$\cos\alpha$	9.853328				
B	8.510797			B	8.510798				
h	1.356172			h	1.638969				
s^2	6.6167			s^2	6.5497				
$\sin^2\alpha$	9.9452			$\sin^2\alpha$	9.6911				
C	1.3393			C	1.3390				
	7.9017				7.5798				
h^2	2.70			h^2	3.278				
D	2.388			D	2.388				
					5.666				
VALUES IN SECONDS									
1st term	+	22.7076		1st term	+	43.5481			
2d term	+	00.80		2d term	+	00.38			
3d term	+			3d term	+				
$-\Delta\phi$		22.7156		$-\Delta\phi$		43.5443			
LOGARITHMS									
s	3.308359			s	3.274843				
$\cos\alpha$	9.537016			$\cos\alpha$	9.853328				
B	8.510797			B	8.510798				
h	1.356172			h	1.638969				
s^2	6.6167			s^2	6.5497				
$\sin^2\alpha$	9.9452			$\sin^2\alpha$	9.6911				
C	1.3393			C	1.3390				
	7.9017				7.5798				
h^2	2.70			h^2	3.278				
D	2.388			D	2.388				
					5.666				
VALUES IN SECONDS									
1st term	+	22.7076		1st term	+	43.5481			
2d term	+	00.80		2d term	+	00.38			
3d term	+			3d term	+				
$-\Delta\phi$		22.7156		$-\Delta\phi$		43.5443			
LOGARITHMS									
s	3.308359			s	3.274843				
$\cos\alpha$	9.537016			$\cos\alpha$	9.853328				
B	8.510797			B	8.510798				
h	1.356172			h	1.638969				
s^2	6.6167			s^2	6.5497				
$\sin^2\alpha$	9.9452			$\sin^2\alpha$	9.6911				
C	1.3393			C	1.3390				
	7.9017				7.5798				
h^2	2.70			h^2	3.278				
D	2.388			D	2.388				
					5.666				
VALUES IN SECONDS									
1st term	+	22.7076		1st term	+	43.5481			
2d term	+	00.80		2d term	+	00.38			
3d term	+			3d term	+				
$-\Delta\phi$		22.7156		$-\Delta\phi$		43.5443			
LOGARITHMS									
s	3.308359			s	3.274843				
$\cos\alpha$	9.537016			$\cos\alpha$	9.853328				
B	8.510797			B	8.510798				
h	1.356172			h	1.638969				
s^2	6.6167			s^2	6.5497				
$\sin^2\alpha$	9.9452			$\sin^2\alpha$	9.6911				
C	1.3393			C	1.3390				
	7.9017				7.5798				
h^2	2.70			h^2	3.278				
D	2.388			D	2.388				
					5.666				
VALUES IN SECONDS									
1st term	+	22.7076		1st term	+	43.5481			
2d term	+	00.80		2d term	+	00.38			
3d term	+			3d term	+				
$-\Delta\phi$		22.7156		$-\Delta\phi$		43.5443			
LOGARITHMS									
s	3.308359			s	3.274843				
$\cos\alpha$	9.537016			$\cos\alpha$	9.853328				
B	8.510797			B	8.510798				
h	1.356172			h	1.638969				
s^2	6.6167			s^2	6.5497				
$\sin^2\alpha$	9.9452			$\sin^2\alpha$	9.6911				
C	1.3393			C	1.3390				
	7.9017				7.5798				
h^2	2.70			h^2	3.278				
D	2.388			D	2.388				
					5.666				
VALUES IN SECONDS									
1st term	+	22.7076		1st term	+	43.5481			
2d term	+	00.80		2d term	+	00.38			
3d term	+			3d term	+				
$-\Delta\phi$		22.7156		$-\Delta\phi$		43.5443			

N.A. Dyer

POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

FIRST ANGLE OF TRIANGLE

11--9362 U. S. GOVERNMENT PRINTING OFFICE: 1959

Above position is on N.A. Datum

N.A. 1927 Datum
40° - 42' - 1317.4
74° - 03' - 537.8

Geodetic positions from transverse Mercator coordinates

State New Jersey

Station

$X = 2,172,500$
 $Y = 690,000$

x	2 172 500.00	log S_g	5.23678416
C	2	log (1200/3937)	9.48401583
$x' (=x-C)$	+ 172 500.00	log (1/R)	1.086
$x'^3/(6\rho_0^2)_g$	- 1.96	log S_m	4.72081085
S_g	+ 172 498.04	cor. arc to sine	- 490
		log S_1	4.72080595
log S_m^2	9.44162170	log A	8.80910000
log C	1.339429	log sec ϕ	0.12042293
log $\Delta\phi$	0.781051	log $\Delta\lambda_1$	3.35032888
		cor. sine to arc	+ 854
y	690 000.00	log $\Delta\lambda$	3.35033742
ϕ' (by interpolation)	40° 43' 39".3677	$\Delta\lambda$	22" 40.4 171
$\Delta\phi$	- 06.0402	λ (central mer.)	74° 40' 00.000
ϕ	40 43 33.328	$\Delta\lambda$	37 20.417
Sec. in meters	1028.0	λ	75 17 20.417 74° 02' 39.539
	(822.7)		927.9

add $\frac{1}{450}$ distortion correction

Station

$X = 2,175,000$
 $Y = 690,000$ (480.2)

x	2 175 000.00	log S_g	5.24308299
C		log (1200/3937)	9.48401583
$x' (=x-C)$	175 000.00	log (1/R)	1.086
$x'^3/(6\rho_0^2)_g$	- 2.04	log S_m	4.72705968
S_g	174 997.96	cor. arc to sine	- 505
		log S_1	4.72705463
log S_m^2	9.45411936	log A	8.50910000
log C	1.339429	log sec ϕ	0.12042261
log $\Delta\phi$	0.793548	log $\Delta\lambda_1$	3.35657724
		cor. sine to arc	+ 879
y	690 000.00	log $\Delta\lambda$	3.35658603
ϕ' (by interpolation)	40° 43' 39".3677	$\Delta\lambda$	2272".9298
$\Delta\phi$	- 06.2165	λ (central mer.)	74° 40' 00.000
ϕ	40 43 33.151	$\Delta\lambda$	37 52.930
	1022.6	λ	75 17 52.930 74° 02' 07.070
	(828.2)		165.9

photo

1408.1

165.9

(over) (1242.2)

Explanation of form:

$$x' = x - C$$

$$S_g = x' - \frac{x'^3}{(6\rho_o^2)_g}$$

$$S_m = \frac{1}{R} \left(\frac{1200}{3937} \right) S_g$$

R = scale reduction factor

ϕ' is interpolated from table of y

$$\Delta\phi = C S_m^2$$

$$\phi = \phi' - \Delta\phi$$

$$\Delta\lambda_1 = S_1 A \sec \phi$$

$$\log S_1 = \log S_m - \text{cor. arc to sine}$$

$$\log \Delta\lambda = \log \Delta\lambda_1 + \text{cor. arc to sine}$$

$$\lambda = \lambda(\text{central mer.}) - \Delta\lambda$$

1/2
5

Geodetic positions from transverse Mercator coordinates

State New Jersey

Station

$x = 2,172,500$
 $y = 700,000$

x	2 172 500.00	log S_g	5 236 78 416
C	2 000 000.00	log (1200/3937)	9.48401583
$x' (=x-C)$	172 500.00	log (1/R)	1 086
$x'^3/(6\rho_0^2)_g$	- 1.96	log S_m	4.72081085
S_g	172 498.04	cor. arc to sine	- 490
		log S_1	4.72080595
log S_m^2	9.44162170	log A	8.50909930
log C	1.339847	log sec ϕ	0.12060213
log $\Delta\phi$	0.781469	log $\Delta\lambda_1$	3.35050738
		cor. sine to arc	+ 855
y	700 000.00	log $\Delta\lambda$	3.35051593
ϕ' (by interpolation)	40° 45' 18".1826	$\Delta\lambda$	2241".3822
$\Delta\phi$	- 06.0460	λ (central mer.)	74° 40' 00".000
ϕ	40 45 12.137	$\Delta\lambda$	37 21.382
sec. in meters.	374.4	λ	74° 02' 38.618 75 17 21.382
	(1476.4)		905.9 905.9 (501.7)

Station

$x = 2,175,000$
 $y = 700,000$

x	2 175 000.00	log S_g	5.24303299
C		log (1200/3937)	9.48401583
$x' (=x-C)$	175 000.00	log (1/R)	1 086
$x'^3/(6\rho_0^2)_g$	- 2.04	log S_m	4.72705968
S_g	174 997.96	cor. arc to sine	- 505
		log S_1	4.72705463
log S_m^2	9.45411936	log A	8.50909930
log C	1.339847	log sec ϕ	0.12060181
log $\Delta\phi$	0.793966	log $\Delta\lambda_1$	3.35675574
		cor. sine to arc	+ 880
y	700000.00	log $\Delta\lambda$	3.35676454
ϕ' (by interpolation)	40° 45' 18".1826	$\Delta\lambda$	2273".8643
$\Delta\phi$	- 06.2225	λ (central mer.)	74° 40' 00".000
ϕ	40 45 11.960	$\Delta\lambda$	37 53.864
sec. in meters.	368.9	λ	74° 03' 06.139 75 17 21.382
	(1481.9)		143.9 (1263.7) over

Explanation of form:

$$x' = x - C$$

$$S_g = x' - \frac{x'^3}{(6\rho_o^2)_g}$$

$$S_m = \frac{1}{R} \left(\frac{1200}{3937} \right) S_g$$

R = scale reduction factor

ϕ' is interpolated from table of y

$$\Delta\phi = C S_m^2$$

$$\phi = \phi' - \Delta\phi$$

$$\Delta\lambda_1 = S_1 A \sec \phi$$

$$\log S_1 = \log S_m - \text{cor. arc to sine}$$

$$\log \Delta\lambda = \log \Delta\lambda_1 + \text{cor. arc to sine}$$

$$\lambda = \lambda(\text{central mer.}) - \Delta\lambda$$

23473

Geodetic positions from transverse Mercator coordinates

State

New Jersey

Station

$X = 2,170,000$
 $Y = 685,000$

x	2 170 000.00	log S_g	5.23044414 ✓
C		log (1200/3937)	9.48401583 ✓
$x' (=x-C)$	170 000.00	log (1/R)	1086 ✓
$x'^3/(6\rho_0^2)_g$	- 1.87	log S_m	4.71447083 ✓
S_g	169 998.13	cor. arc to sine	- 476 ✓
		log S_1	4.71446607 ✓
log S_m^2	9.42894166 ✓	log A	8.50910034 ✓
log C	1.339220 ✓	log sec ϕ	0.12033372 ✓
log $\Delta\phi$	0.768162 ✓	log $\Delta\lambda_1$	3.34390013 ✓
		cor. sine to arc	+ 829 ✓
y	685 000.00	log $\Delta\lambda$	3.34390842 ✓
ϕ' (by interpolation)	40° 42' 49.9602	$\Delta\lambda$	2207.5392 ✓
$\Delta\phi$	- 05.8636	λ (central mer.)	74° 40' 00.000 ✓
ϕ	40 42 44.0974	$\Delta\lambda$	36.47.539 ✓
	1360.2 m	λ	74° 03' 12.461 ✓
	(490.6)		292.4

Station

$X = 2,175,000$
 $Y = 685,000$

x	2 175 000.00	log S_g	5.24303299 ✓
C		log (1200/3937)	9.48401583 ✓
$x' (=x-C)$	175 000.00	log (1/R)	1086 ✓
$x'^3/(6\rho_0^2)_g$	- 2.04	log S_m	4.72705968 ✓
S_g	174 997.96	cor. arc to sine	- 605 ✓
		log S_1	4.72705463 ✓
log S_m^2	9.45411936 ✓	log A	8.50910035 ✓
log C	1.339220 ✓	log sec ϕ	0.12033308 ✓
log $\Delta\phi$	0.793339 ✓	log $\Delta\lambda_1$	3.35648806 ✓
		cor. sine to arc	+ 879 ✓
y	685 000.00	log $\Delta\lambda$	3.35649685 ✓
ϕ' (by interpolation)	40° 42' 49.9602	$\Delta\lambda$	2272.4632 ✓
$\Delta\phi$	- 06.2135	λ (central mer.)	74° 40' 00.000 ✓
ϕ	40 42 43.747	$\Delta\lambda$	37 52.463 ✓
	1349.4 m	λ	74° 17' 52.463 ✓
	(501.4)		74° 02' 07.537 ✓

(1231.6) (over)

(M-29)

Explanation of form:

$$x' = x - C$$

$$S_g = x' - \frac{x'^3}{(6\rho_o^2)_g}$$

$$S_m = \frac{1}{R} \left(\frac{1200}{3937} \right) S_g$$

R = scale reduction factor

ϕ' is interpolated from table of y

$$\Delta\phi = C S_m^2$$

$$\phi = \phi' - \Delta\phi$$

$$\Delta\lambda_1 = S_1 A \sec \phi$$

$$\log S_1 = \log S_m - \text{cor. arc to sine}$$

$$\log \Delta\lambda = \log \Delta\lambda_1 + \text{cor. arc to sine}$$

$$\lambda = \lambda(\text{central mer.}) - \Delta\lambda$$

Geodetic positions from Lambert coordinates

State Long Island Station Intersection $x = 1,985,000$
 $y = 175,000$

x	1,985,000	$R_b + A$	24,462,545.30
C	2	y	175,000
$x' (= x - C)$	-15,000	$R_b + A - y$	24,287,545.30
$\tan \theta$	0.0006176004950	R	
θ	$\left\{ \begin{array}{l} 0^\circ 02' 07.389230 \\ \text{"} \end{array} \right.$	y	175,000
$\frac{\theta}{\ell} (= \Delta \lambda)$		y''	4.63
		y'	174,995.37
λ (central mer.)	74°	ϕ (by interpolation)	40° 42' 21.061
$-\Delta \lambda$	3 14.760		
λ	74 03 14.760		
sec. in meters	346.5		649.6

1062.0

Station $x = 1,990,000$
 $y = 175,000$ (1201.2)

x	1,990,000	$R_b + A$	24,462,545.30
C	2	y	175,000
$x' (= x - C)$	-10,000	$R_b + A - y$	24,287,545.30
$\tan \theta$	0.0004117336633	R	
θ	$\left\{ \begin{array}{l} 0^\circ 1' 24.926159 \\ \text{"} \end{array} \right.$	y	175,000
$\frac{\theta}{\ell} (= \Delta \lambda)$		y''	2.06
		y'	174,997.94
λ (central mer.)	74°	ϕ (by interpolation)	40° 42' 21.087
$-\Delta \lambda$	2 09.840		
λ	74 02 09.840		
sec. in meters	231.0		650.4

(1177.5)

(1200.4)

$$\tan \theta = \frac{x - C}{R_b + A - y}$$

$$\Delta \lambda = \frac{\theta}{\ell}$$

$$\lambda = \lambda \text{ (central mer.)} - \Delta \lambda$$

$$R = (R_b + A - y) \sec \theta$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

C is constant added to x' in computation
of coordinates

R_b is map radius of lowest parallel

A is value of y' for R_b ; in most cases it is zero

ϕ is interpolated from table of y'

Geodetic positions from Lambert coordinates

State Long Island Station Intersection $X = 1,987,500$
 $Y = 190,000$

x	1,987,500	$R_b + A$	24,462,545.30
C	2	y	190,000
$x' (= x - C)$	-12,500	$R_b + A - y$	24,272,545.30
$\tan \theta$	0.0005149851343	R	
θ	$\left\{ \begin{array}{l} 0^\circ 1' 46''.223367 \end{array} \right.$	y	190,000
$\frac{\theta}{\rho} (= \Delta \lambda)$		y''	- 3.22
λ (central mer.)	$74^\circ \quad ' \quad ''$	y'	189,996.78
$-\Delta \lambda$	2 42.401	ϕ (by interpolation)	$40^\circ 44' 49''.294$
λ	74 02 42.401		
Sec. in meters	994.7 ✓		1520.5

Station $X = 1,990,000$ (330.3)
 $Y = 190,000$

x	1,990,000	$R_b + A$	24,462,545.30
C	2	y	190,000
$x' (= x - C)$	-10,000	$R_b + A - y$	24,272,545.30
$\tan \theta$	0.0004119881074	R	
θ	$\left\{ \begin{array}{l} 0^\circ 1' 24.978642 \\ \text{"} \end{array} \right.$	y	190,000
$\frac{\theta}{\ell} (= \Delta \lambda)$		y''	2.06
λ (central mer.)	$74^\circ \quad ' \quad "$	y'	189,997.94 ✓
$-\Delta \lambda$	2 09.920	ϕ (by interpolation)	$40^\circ 44' 49.306''$ ✓
λ	$74^\circ 02' 09.920''$		
Sec. in meters	2332.7		1520.9

$$\tan \theta = \frac{x - C}{R_p + A - y}$$

$$\Delta\lambda = \frac{\theta}{f}$$

$$\lambda = \lambda(\text{central mer.}) - \Delta\lambda$$

$$R = (R_b + A - y) \sec \theta$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

C is constant added to x' in computation of coordinates

R_b is map radius of lowest parallel

A is value of y' for R_b ; in most cases it is zero

ϕ is interpolated from table of y'

Geodetic positions from Lambert coordinates

State Long Island Station Intersection $x = 1,987,500$
 $y = 180,000$

x	1,987,500	$R_b + A$	24,462,545.30
C	2	y	180,000
$x' (= x - C)$	-12,500	$R_b + A - y$	24,282,545.30
$\tan \theta$	0.0005147730539	R	
θ	$\left\{ \begin{array}{l} 0^\circ 1' 46.179555'' \\ \end{array} \right.$	y	180,000
$\frac{\theta}{\ell} (= \Delta \lambda)$		y''	- 3.22
λ (central mer.)	74° ' "	y'	179,996.78
$-\Delta \lambda$	+ 2 42.334	ϕ (by interpolation)	40° 43' 10.482
λ	74 02 42.334		
sec in meters	99.346		323.3

(414.7)

Station $x = 1,990,000$ (1527.5)
 $y = 180,000$

x	1,990,000	$R_b + A$	24,462,545.30
C	2	y	180,000
$x' (= x - C)$	-10,000	$R_b + A - y$	24,282,545.30
$\tan \theta$	0.0004118184431	R	
θ	$\left\{ \begin{array}{l} 0^\circ 1' 24.943647'' \\ \end{array} \right.$	y	180,000
$\frac{\theta}{\ell} (= \Delta \lambda)$		y''	- 2.06
λ (central mer.)	74° ' "	y'	179,997.94
$-\Delta \lambda$	2 09.867	ϕ (by interpolation)	40° 43' 10.493
λ	74 02 09.867		
sec in meters	231.6		323.7

(1176.7)

(1527.1)

$$\tan \theta = \frac{x - C}{R_b + A - y}$$

$$\Delta \lambda = \frac{\theta}{\ell}$$

$$\lambda = \lambda (\text{central mer.}) - \Delta \lambda$$

$$R = (R_b + A - y) \sec \theta$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

C is constant added to x' in computation
of coordinates

R_b is map radius of lowest parallel

A is value of y' for R_b ; in most cases it is zero

ϕ is interpolated from table of y'

Geodetic positions from Lambert coordinates

State Long Island

Station _____

{ 1,994,000
192,000

x	1,994,000	$R_b + A$	24,462,545.30
C	2	y	192,000
$x' (= x - C)$	-6,000	$R_b + A - y$	24,270,545.30
$\tan \theta$	0.000 247213234	R	
θ	{ 0° 0' 50".991389	y	192,000
$\frac{\theta}{\ell} (= \Delta \lambda)$	77.9587	y''	- 0.74
λ (central mer.)	74° 00' 00".000	y'	191,999.26
$-\Delta \lambda$	1 17.959	ϕ (by interpolation)	40° 45' 09".081 ✓
λ	74 01 17.959		
see in	421.3		280.11

185° 0.8
1407.6

Station _____

x		$R_b + A$	
C		y	
$x' (= x - C)$		$R_b + A - y$	
$\tan \theta$		R	
θ	{ 0 ' "	y	
$\frac{\theta}{\ell} (= \Delta \lambda)$		y''	-
λ (central mer.)	0 ' "	y'	
$-\Delta \lambda$		ϕ (by interpolation)	0 ' "
λ			

$$\tan \theta = \frac{x - C}{R_b + A - y}$$

$$\Delta \lambda = \frac{\theta}{\ell}$$

$$\lambda = \lambda (\text{central mer.}) - \Delta \lambda$$

$$R = (R_b + A - y) \sec \theta$$

$$y'' = 2R \sin^2 \frac{\theta}{2}$$

$$y' = y - y''$$

C is constant added to x' in computation
of coordinates

R_b is map radius of lowest parallel

A is value of y' for R_b ; in most cases it is zero

ϕ is interpolated from table of y'

GEODETIC POSITIONS FROM TRANSVERSE MERCATOR COORDINATES

STATE New Jersey

STATION { 2,178,000
700,000

x	2,178,000	$\log S_e$	5.25041475
K	2	$\log (1200/3937)$	9.48401583
$x' (=x-K)$	+ 178,000	$\log (1/R)$	1.086
$x'^3/(6\rho_e^2)_e$	2.15	$\log S_m$	4.73444144
S_e	+ 177,997.85	cor. arc to sine	522
		$\log S_1$	4.73443622
$3 \log x'$	15.75126	$\log A$	8.50909931
$\log 1/(6\rho_e^2)_e$	4.58102	$\log \sec \phi$	0.12060142
$\log x'^3/(6\rho_e^2)_e$	0.33228	$\log \Delta\lambda_1$	3.36413695
		cor. sine to arc	+ 910
$\log S_m^2$	9.468883	$\log \Delta\lambda$	3.36414605
$\log C$	1.3398447	$\Delta\lambda$	+ 2312.8424
$\log \Delta\phi$	0.808730		
y	700,000		
ϕ' (by interpolation)	40° 45' 18.183	λ (central mer.)	74° 40' "
$\Delta\phi$	6.438	$\Delta\lambda$	38 32.842
ϕ	46.21	λ	74 01 27.158
	40 45 11.745		637.0

sec in meters 362.3
Explanation of form:

$$x' = x - K$$

$$\begin{matrix} 1850.8 \\ 1407.6 \end{matrix}$$

$$S_e = x' - \frac{x'^3}{(6\rho_e^2)_e}$$

$$S_m = \frac{1}{R} \left(\frac{1200}{3937} \right) S_e$$

R = scale reduction factor

ϕ' is interpolated from table of y

$$\Delta\phi = C S_m^2$$

$$\phi = \phi' - \Delta\phi$$

$$\Delta\lambda_1 = S_1 A \sec \phi$$

$$\log S_1 = \log S_m - \text{cor. arc to sine}$$

$$\log \Delta\lambda = \log \Delta\lambda_1 + \text{cor. arc to sine}$$

$$\lambda = \lambda \text{ (central mer.)} - \Delta\lambda$$

Plane coordinates on Lambert projection

(for Datum Station) State Long Island Station Castle Stevens (N.J.) 1875

$\phi = 40^{\circ} 44' 41.152''$ $\lambda = 74^{\circ} 01' 28.040''$

Tabular difference of R for 1" of $\phi = 101.20167$

R (for min. of ϕ)	24,277,537.19	y' (for min. of ϕ)	185,008.11
Cor. for sec. of ϕ	- 4164.65	Cor. for sec. of ϕ	+ 4164.65
R	24,273,372.54	y'	189,172.76
		y'' (= $2R \sin^2 \frac{\theta}{2}$)	+ 0.95
θ (for min. of λ)	- $0^{\circ} 00' 39.24493''$	y	189,173.71
Cor. for sec. of λ	- 18.34046		
θ	- 57.58539	$\frac{\theta}{2}$	$0^{\circ} 00' 28.792695''$
θ''	For machine computation		For machine computation
log θ''	1.76031231	log θ''	1.76031231
S for θ	4.68557486	colog 2	9.69897000
log sin θ	6.44588717	S for $\frac{\theta}{2}$	4.68557487
log R	7.38513012	log sin $\frac{\theta}{2}$	6.14485718
log x'	3.83101729	R sin $\frac{\theta}{2}$	2.28971436
x'	R sin θ	log sin ² $\frac{\theta}{2}$	7.38513012
	2,000,000.00	log R	0.30103000
x	1,993,223.32	log 2	9.97587448
		log y''	

$$x = 2,000,000.00 + R \sin \theta$$

$$y = y' + 2R \sin^2 \frac{\theta}{2}$$

y' = the value of y on the central meridian for the latitude of the station

S = log of ratio for reducing arc expressed in seconds to sine

(see log tables)

R, y', and θ are given in special tables

Not
only
for
plotting
E. S. B.
Aug. 19, 1936

REVIEW OF AIR PHOTO COMPILATION T-5450 (1934)
Scale 1:5,000

Comparison with Graphic Control Surveys

T-6127 (1934), 1:10,000

Survey T-6127 was made during September, October 1934 about one month before the photographs of this area were taken. T-6127 shows all waterfront detail between Ellis Island and Constable Point.

The common area between T-6127 and this compilation is small. Over this area all detail on T-6127 is also shown on this compilation. The piles and dolphins were transferred to the compilation in this office.

There are differences in location of detail between this compilation (scale 1:5,000) and T-6127 (scale 1:10,000) of from 2 to 15 meters. The large difference of 15 meters is for a small section only. The compilation is well controlled, is on a larger scale and is accepted after examination in this office. A note has been placed on T-6127.

Comparison with Previous Topographic Surveys

T- 16	(1837), 1:10,000	
T- 17	(1837)	"
T- 18	(1837)	"
T-482	(1855)	" Jersey City to Caven Point
T-484	(1855)	" Jersey City and vicinity
T-610a	(1859)	" Hoboken and Jersey City
T-610b	(1873)	" Castle Point to Ellis Island
T-610c	(1875)	" Guttenberg to Bulls Ferry
T-662	(1857)	" Vicinity of Communipaw

Comparison with the old surveys listed above shows that this compilation is complete and adequate to supersede the sections of those surveys which it covers. There have been numerous and extensive changes due to harbor developments since the time these surveys were made. No detailed discussion will be made for them.

T-1573 (1885), 1:5,000

Survey T-1573 covers the Hudson River between Castle Point and Guttenberg. Numerous large changes have occurred since 1885. The compilation is adequate in order to supersede T-1573 over the common area.

T-1575 (1885), 1:5,000

Survey T-1575 is in the vicinity of Ellis Island. Over the common area there have been many changes of detail. The large pier

located at lat. $40^{\circ} 42.25'$, long. $74^{\circ} 02.1'$ has been removed. T-1575 is superseded by this compilation over the common area.

T-1578 (1885), 1:5,000

Survey T-1578 covers the Hudson River from latitude $40^{\circ} 42.5'$ to Castle Point. There have been numerous changes of piers and docks in this area since 1885. T-1578 is superseded by this compilation, over the common area.

T-2323 (1889), 1:10,000, Chart 369⁴
 T-3150 (1911), " " "
 T-3226 (1911), " " "

The surveys T-2323, T-3150, T-3226 are revision surveys which have been made directly on current editions of chart 369⁴. Revised detail is shown in red. The compilation is adequate in order to supersede the portions of these surveys which it covers.

There are no new hydrographic surveys covering this area.

Comparison with charts Nos. 745, 369, 1215.

All current landmarks shown on charts 745 and 369 within this area are shown on this compilation. These landmarks are contained in chart letter 176 (1935). No additional landmarks have been recommended in this area. There are no lights or other non-floating aids to navigation in this area.

The extent of the high water line at the inshore ends of the ferry slips shown on this compilation at lat. $40^{\circ} 44.1'$, long. $74^{\circ} 01.6'$, lat. $40^{\circ} 43.6'$, long. $74^{\circ} 01.8'$ and lat. $40^{\circ} 42.4'$, long. $74^{\circ} 02.1'$ cannot be determined from the photographs on account of these ends falling under roof coverings. The compilation in each case shows the line around the slips to the point where they join the projection of the roof on the plane of the slip. The slips are joined by the roof projections.

A portion of the building on Pier 16 at lat. $40^{\circ} 45.3'$, long. $74^{\circ} 01.5'$ has been removed.

Two buildings shown at lat. $40^{\circ} 45.0'$, long. $74^{\circ} 01.5'$ have been removed.

Two small buildings shown at lat. $40^{\circ} 43.55'$, long. $74^{\circ} 02.3'$ have been removed.

The shaded city block at lat. $40^{\circ} 43.2'$, long. $74^{\circ} 02'$ on chart 745 should be left blank as there are no buildings in this block.

Plane Coordinate Grids

Both the Long Island and the New Jersey Systems of Plane Coordinates are shown on this compilation. These coordinates, marked at intervals of 2,000 feet, were plotted in this office from geographic positions of intersections computed in the Division of Geodesy. Computations are attached at back of this report. Grids plotted by

L. A. McGann.

and plotting checked by B. G. Jones.

Intermediate points plotted on projection machine

Sept. 16, 1936.

Reviewed by L. A. McGann.

Lemuel A. McGann

B. G. Jones

The projection for this compilation was hand drawn. The lines are very heavy in places and in several places vary up to 0.1 to 0.3 mm from correct position. The grid has been worked slightly to fit as nearly as possible with the projection.

B. G. J.

REVIEW OF AIR PHOTO COMPILATION NO. *T-5450*.

Chief of Party: J.C. Partington

Compiled by: See STATISTICS
SHEET

Project: HT-175

Instructions dated: Mar. 14, 1934.

1. The charts of this area have been examined and topographic information necessary to bring the charts up to date is shown on this compilation. (Par. 16a, b, c, d, e, g and i; 26; and 64) ✓
2. Change in position, or non-existence of wharfs, lights, and other topographic detail of particular importance to navigation which affect the chart, is discussed in the descriptive report. (Par. 26; and 66 g, n)
*There are no large changes in position of wharfs.
 There are no lights or other non-floating aids to navigation within the area of this compilation.*
3. Ground surveys by plane table, sextant, or theodolite have been used to supplement the photographic plot where necessary to obtain complete information, and all such surveys are discussed in the descriptive report. (Par. 65; and 66 d, e)
 3 point theodolite fix used as control and included with descriptive report. *Survey T-6127 covers a small area of this compilation at its southern end. T-6127 (1934) G.C.S.*
4. Blue-prints and maps from other sources which were transmitted by the field party contain sufficient control for their application to the charts. (Par. 28)
 Blue-prints used as an aid in detailing sheet and for names only.
5. Differences between this compilation and contemporary plane table and hydrographic surveys have been examined and rectified in the field before forwarding the compilations to the office and are discussed in the descriptive report.
 No contemporary surveys on hand in this party for comparison.
T-6127 was being finished at the time the photos were taken, however.
6. The control and adjustment of the photo plot are discussed in the descriptive report. Unusual or large adjustments are discussed in detail and limits of the area affected are stated. (Par. 12b; ~~44~~; and 66 c, h, i) ✓
7. High water line on ~~marshy and mangrove~~ coast is clear and adequate for chart compilation. (Par. 16a, 43, and ~~44~~) ✓
No sand beach in this area.

NOTE: Strike out paragraphs, words or phrases not applicable and modify those requiring it. Paragraph numbers refer to those in the Topographic Manual. Refer also to the pamphlet "Notes on the Compilation of Planimetric Line Maps from Five Lens Air Photographs."

8. The representation of low water lines, reefs, coral reefs and rocks, and legends pertaining to them is satisfactory. (Par. 36, ~~37~~, ~~38~~, 39, 40, ~~41~~)

No low water lines shown.

9. Recoverable objects have been located and described on Form 524 in accordance with circular 30, 1933, circular letter of March 3, 1933, and circular 31, 1934. (Par. 29, 30, and 57)

Cards form 524 included in this report. and filed under compilation No. T-5450.

10. A list of landmarks was furnished on Form 567 and instructions in the Director's letter of July 16, 1934, Landmarks for Charts, complied with. (Par. 16d, e; and 60)

List of Landmarks previously submitted by R.W.Woodworth.

See chart Letter 176 (1933).

11. All bridges shown on the compilation are accompanied by a note stating whether fixed or draw, clearance, and width of draw if a draw bridge. Additional information of importance to navigation is given in the descriptive report. (Par. 16c)

No bridges of importance to navigation in the area of this sheet.

12. Geographic names are shown on the overlay tracing. The accepted local usage of new names has been determined and they are listed in the report, together with a general statement as to source of information and a specific statement when advisable. Complete discussion of place names differing from the charts and from the U. S. G. S. Quadrangles is given in the descriptive report, together with reasons for recommendations made. (Par. 64, and 66k)

13. The geographic datum of the compilation is N.A. 1927 and the reference station is correctly noted.

14. Junctions with adjoining compilations have been examined and are in agreement. (Par. 66j)

15. The drafting is satisfactory and particular attention has been given the following:

1. Standard symbols authorized by the Board of Surveys and Maps have been used throughout except as noted in the report. ✓

2. The degrees and minutes of Latitude and Longitude are correctly marked. ✓

3. All station points are exactly marked by fine ✓✓
black dots.
 4. Closely spaced lines are drawn sharp and clear ✓✓
for printing.
 5. Topographic symbols for similar features are of ✓✓
uniform weight.
 6. All drawing has been retouched where partially ✓✓
rubbed off.
 7. Buildings are drawn with clear straight lines ✓✓
and square corners where such is the case on
the ground.
- (Par. 34, 35, 36, ~~37~~, ~~38~~, 39, 40, ~~41~~, 42, 43, ~~44~~, 45, 46, 48)

16. No additional surveying is recommended at this time.

17. Remarks:

18. Examined and approved;

J. C. Partington
Chief of Party

19. Remarks after review in office:

See following pages immediately preceding

Reviewed in office by: *Leonard A. McGuire* Sept. 16, 1936.

Examined and approved:

C. H. Green
Chief, Section of Field Records
L. O. Lobbut
Chief, Division of Charts

Frederick L. Peacock
Chief, Section of Field Work
W. H. Hulse
Chief, Division of Hydrography
and Topography.